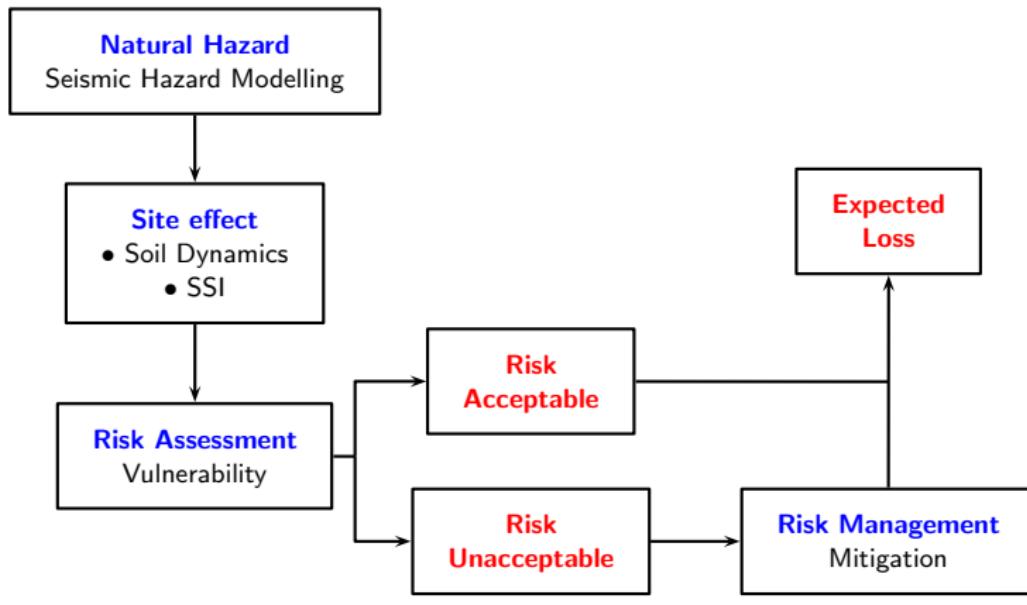


# Coupled elasto-plastic dynamic response of dams

Fernando Lopez-Caballero  
Marc Kham  
Sami Kaloun

# Context

## Earthquake loss estimation



# Global approach

- ▶ Need for understanding mechanisms controlling induced damage in earthquake loss estimation (e.g. soil foundation, structures, dams, ...);
  - ▶ Improve and validate traditional approaches and evaluation methods;
  - ▶ Take into account the non linear soil behaviour;
  - ▶ Use of numerical methods in order to facilitate the comprehension of the global problem via parametric analyses;
- 
- ▶ *Various uncertainties on the material properties, loading parameters and scenarios will be considered;*
  - ▶ *Probabilistic analyses as a complement of conventional deterministic analyses will be used.*

# Outline

Recorded signals

ECP's numerical tool

Numerical model

Conclusions

# Outline

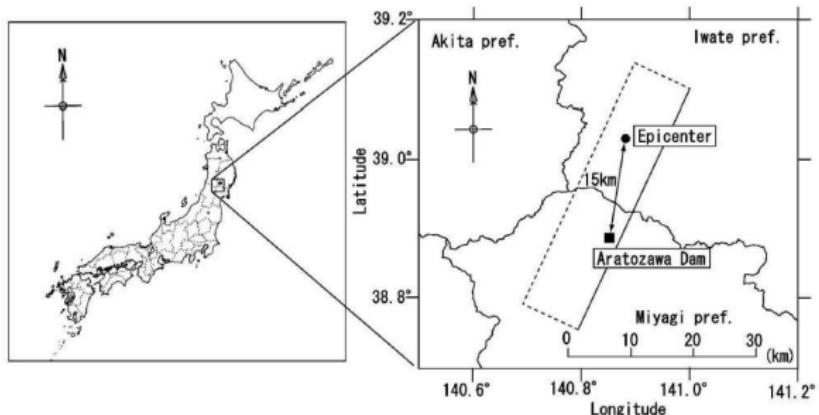
Recorded signals

ECP's numerical tool

Numerical model

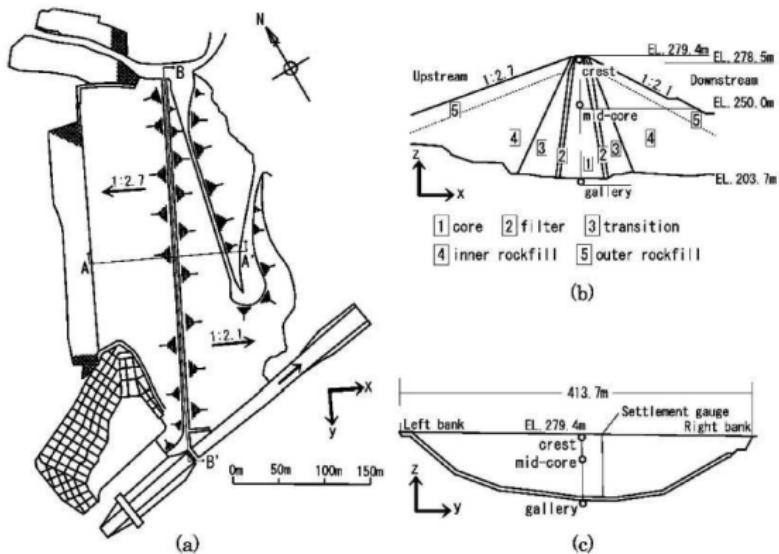
Conclusions

# Aratozawa Dam



The 2008 Iwate-Miyagi Nairiku earthquake [Ohmachi and Taharz, 2011]

# Aratozawa Dam



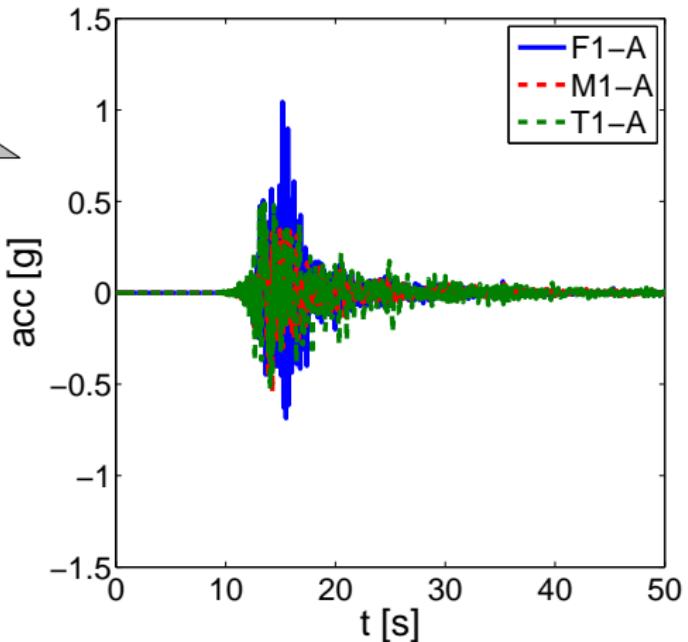
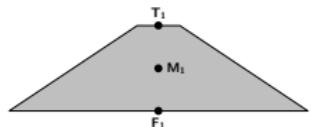
Plan and cross sections of the Aratozawa dam [Ohmachi and Taharz, 2011]

# Recorded signals

	Event	Year	Location	PGA [cm/s <sup>2</sup> ]
<b>Southern Akita Pref</b>	1996	F1-A	<b>28</b>	
		T1-A	<b>105</b>	
Northern Miyagi Pref	1996	F1-A	33	
		T1-A	114	
Northern Miyagi Pref	1996	F1-A	30	
		T1-A	87	
Northern Miyagi Pref	2003	F1-A	113.5	
		T1-A	365	
<b>Southern Iwate Pref</b>	<b>2008</b>	F1-A	<b>1023.8</b>	
		T1-A	<b>525.3</b>	
Far E Off Miyagi Pref	2011	F1-A	102	
		T1-A	290.3	

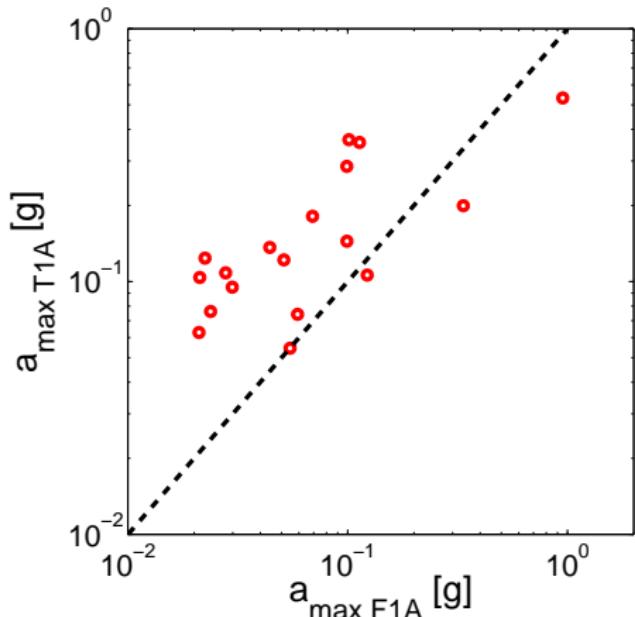
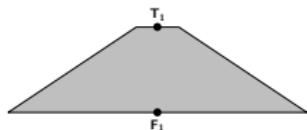
19 earthquake records tested

# Recorded signals

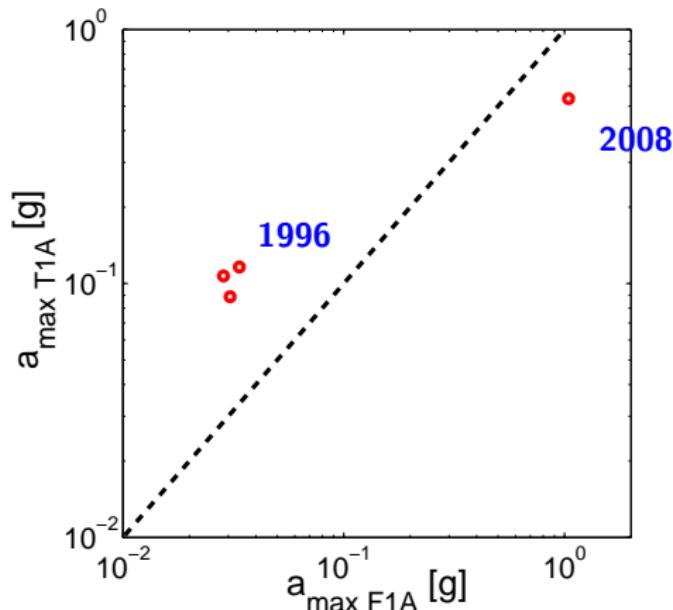
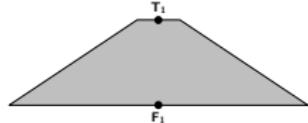


signals in A direction - 2008 Iwate-Miyagi Nairiku Earthquake

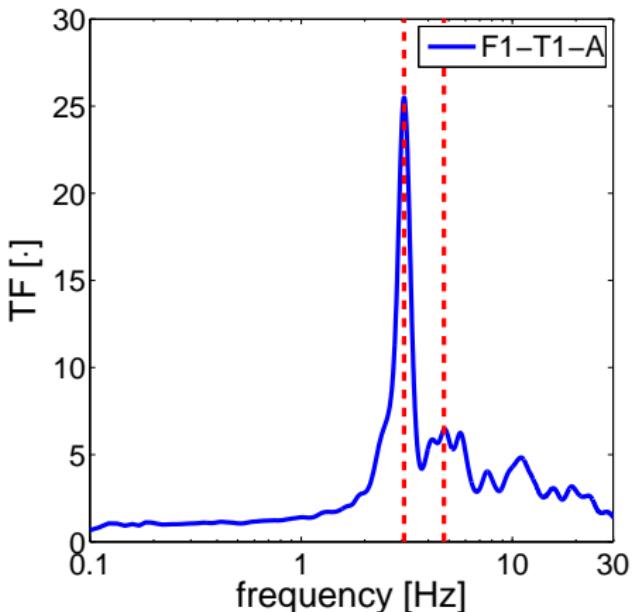
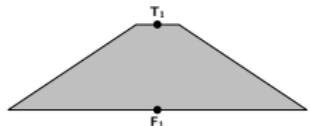
# Recorded signals



# Recorded signals

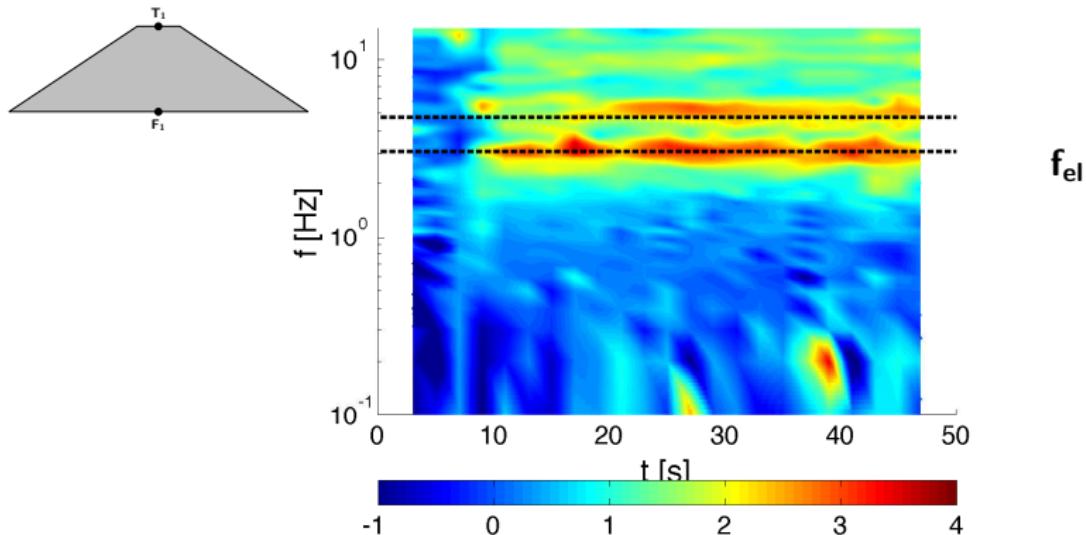


## Recorded signals



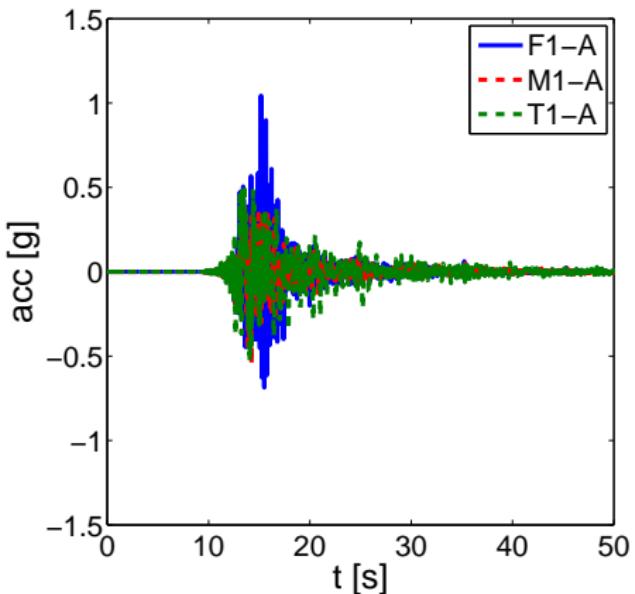
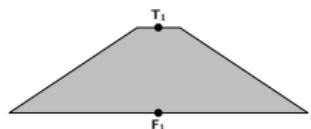
spectral ratio F1-T1- PGA = 30cm/s<sup>2</sup> - 1996 Northern Miyagi Pref  
 $f = 3.06 \text{ and } 4.74 \text{ Hz}$

# Recorded signals



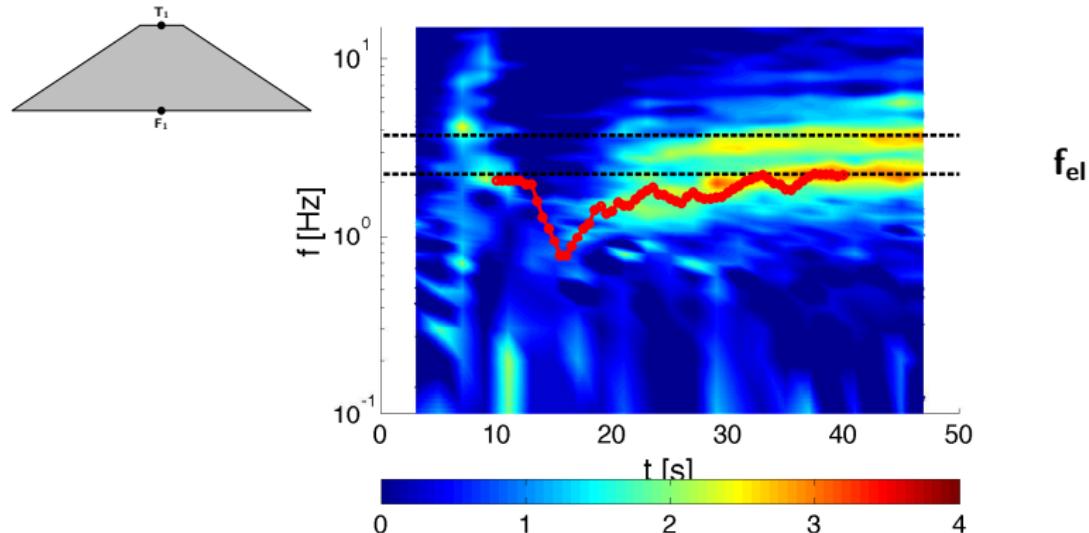
Short Time Fourier Transform (STFT) spectral ratio  $F_1 - T_1$   
3.1 and 4.7 Hz - 1996 Northern Miyagi Pref.

# Recorded signals



Short Time Fourier Transform (STFT) spectral ratio F1-T1  
2008 Southern Iwate Pref

# Recorded signals



Short Time Fourier Transform (STFT) spectral ratio  $F_1-T_1$   
 $(\approx 0.8)$  2.2 and 3.7 Hz - 2008 Southern Iwate Pref

# Recorded signals

Event	PGA [cm/s <sup>2</sup> ]	f <sub>1</sub> [Hz]	f <sub>2</sub> [Hz]
Southern Akita Pref	28	2.91	4.71
Northern Miyagi Pref	33	3.01	5.21
Northern Miyagi Pref	30	3.06	4.74
Southern Iwate Pref	1023.8	2.22*	3.70*

\* Computed between 35-50s.

# Outline

Recorded signals

**ECP's numerical tool**

Numerical model

Conclusions

# GEFDyn & Code\_Aster - ECP's numerical tool

## The ECP's elastoplastic multi-mechanism model

[Aubry et al., 1982, Hujeux, 1985]

- ▶ The model is written in terms of effective stress,
- ▶ Coulomb type failure criterion,
- ▶ Critical state concept,
- ▶ Deviatoric primary yield surface of the  $k$  plane:  $f_k(\sigma, \varepsilon_v^P, r_k) = q_k - \sin \phi'_{pp} \cdot p'_k \cdot F_k \cdot r_k$   
 $F_k = 1 - b \ln \left( \frac{p'_k}{p_c} \right)$  and  $p_c = p_{co} \exp(\beta \varepsilon_v^P)$
- ▶ Progressive friction mobilization with shear:  $r_k = r_k^{el} + \frac{\int_0^t \overline{\varepsilon^P} dt}{a + \int_0^t \overline{\varepsilon^P} dt}$   
 $a = a_1 + (a_2 - a_1) \alpha_k(r_k)$
- ▶ Roscoe's dilatancy law
- ▶ Isotropic yield surface:  $f_{iso} = |p'| - d \cdot p_c \cdot r_{iso}$

# GEFDyn & Code\_Aster - ECP's numerical tool

## Classification of the Elastoplastic model parameters

[Lopez-Caballero et al., 2003]

	Directly measured *	Not-Directly measured
Elastic	$K_{ref}$ , $G_{ref}$ $n_e$ , $p_{ref}$	
Critical State and Plasticity	$\phi'_{pp}$ , $\beta$ $p_{co}$ , $d$	$b$
Flow Rule and Isotropic hardening	$\psi$	$a_1$ , $a_2$ , $\alpha_\psi$ , $m$ , $c_{mon}$
Threshold domains		$r^{ela}$ , $r^{hys}$ $r^{mob}$ , $r_{iso}^{ela}$

\* From : Triaxial, Resonant column, CPT, oedometric tests among others

# Outline

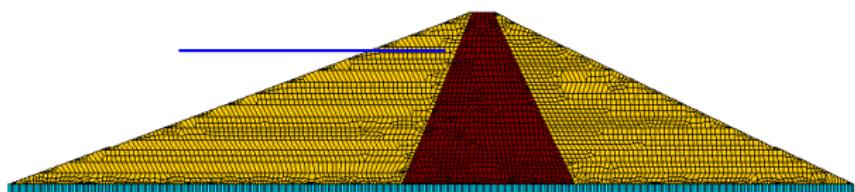
Recorded signals

ECP's numerical tool

**Numerical model**

Conclusions

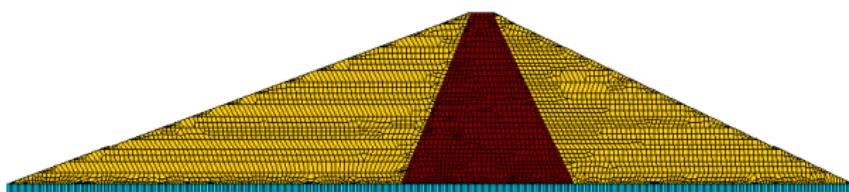
# Numerical model



- ▶ Construction stage of the dam and seismic loading,
- ▶ Two approaches for Pore-water pressure generation in this study,\*
  - ▶ Decoupled behaviour for the core and the upstream rockfill (**effective stress**),
  - ▶ **Coupled behaviour** for the core and the upstream rockfill,
- ▶ Dry condition is supposed for the downstream rockfill (total stress),

\* details in [Montoya-Noguera and Lopez-Caballero, 2016]

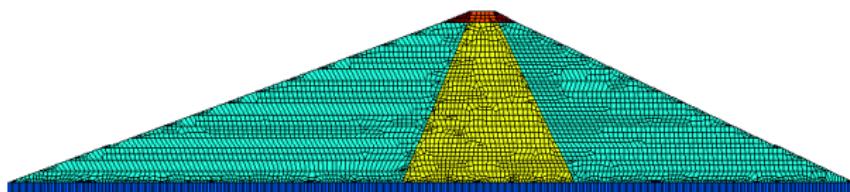
# Numerical model



- ▶ Core → non-linear elasto plastic model (ECP model)
- ▶ Core filter → non-linear elasto plastic model (ECP model)
- ▶ Rockfill → non-linear elasto plastic model (ECP model)
- ▶ Bedrock → infinitely rigid with absorbing elements \*

\* details in [\[Montoya-Noguera, 2016\]](#)

# Numerical model

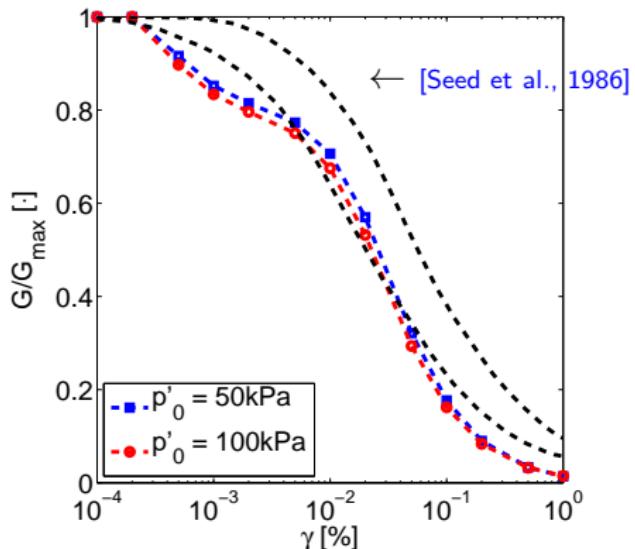


- ▶ Core →  $V_s = 220 \cdot z^{0.35} *$
- ▶ Core filter →  $V_s = 220 \cdot z^{0.35} *$
- ▶ Rockfill →  $V_s = 250 \cdot z^{0.2} *$
- ▶ Bedrock → infinitely rigid with absorbing elements

\* adapted from [\[Ohmachi and Taharz, 2011\]](#)

# Numerical model

Core and Core filter behaviour :

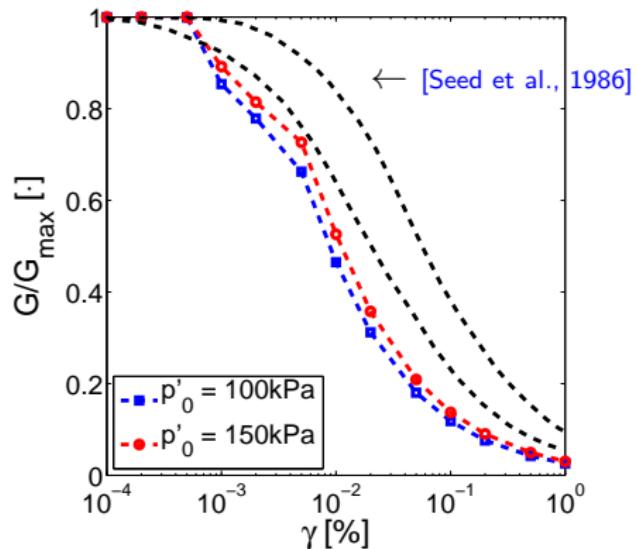


Simulated  $G/G_{max} - \gamma$  curves

Remark : These curves are not an input of the model.

# Numerical model

Rockfill behaviour :

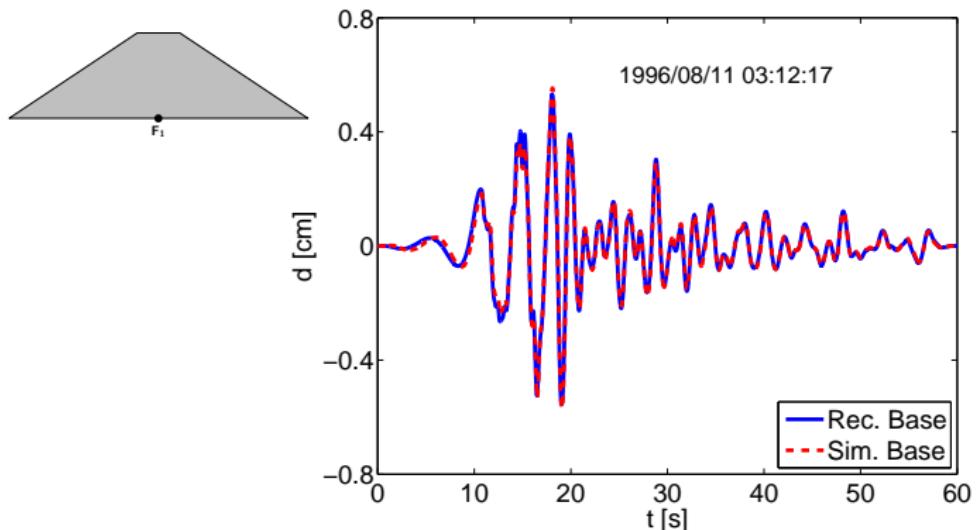


Simulated  $G/G_{max} - \gamma$  curves

Remark : These curves are not an input of the model.

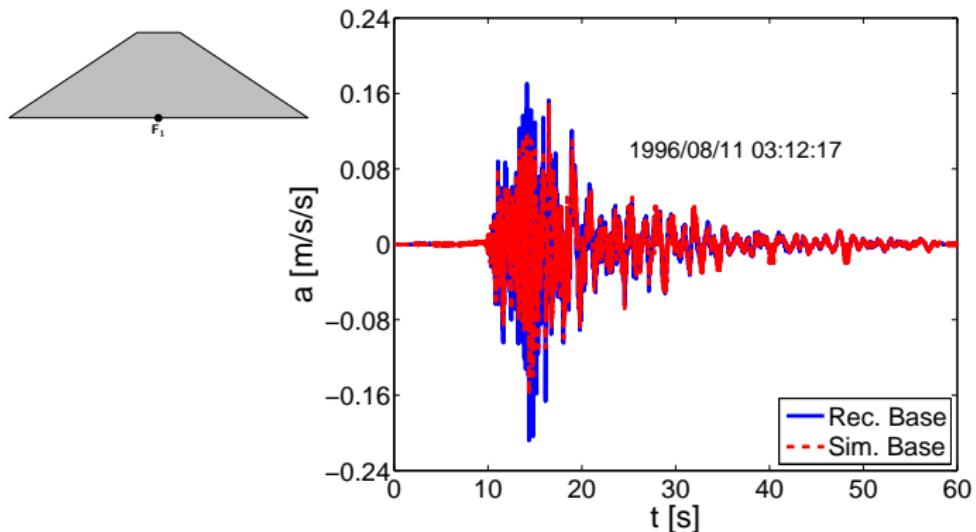
# Numerical model

## Southern Akita Pref - 1996



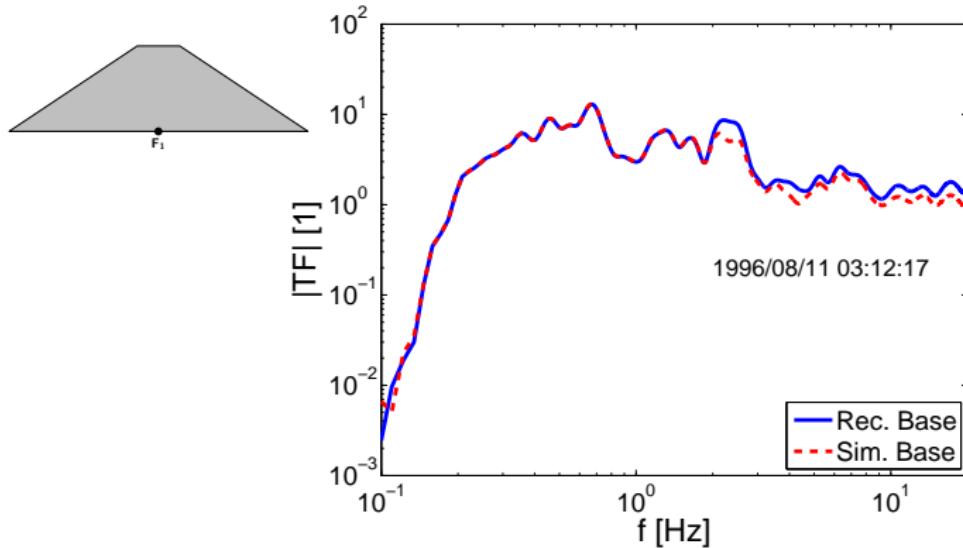
# Numerical model

## Southern Akita Pref - 1996



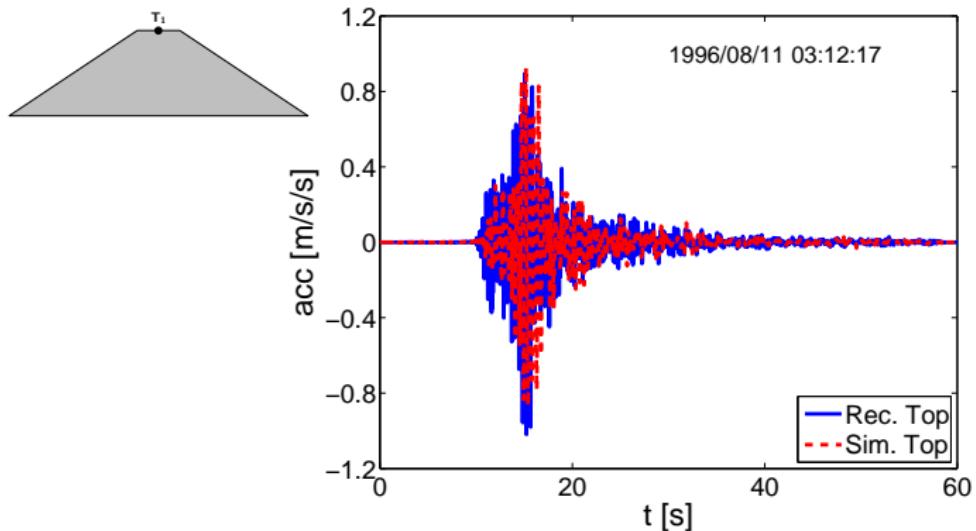
# Numerical model

Southern Akita Pref - 1996



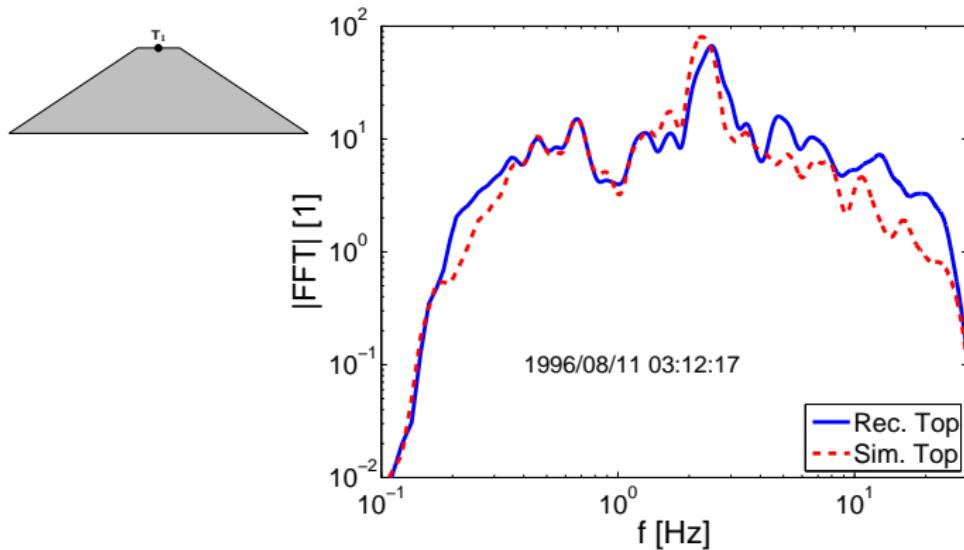
# Numerical model

## Southern Akita Pref - 1996



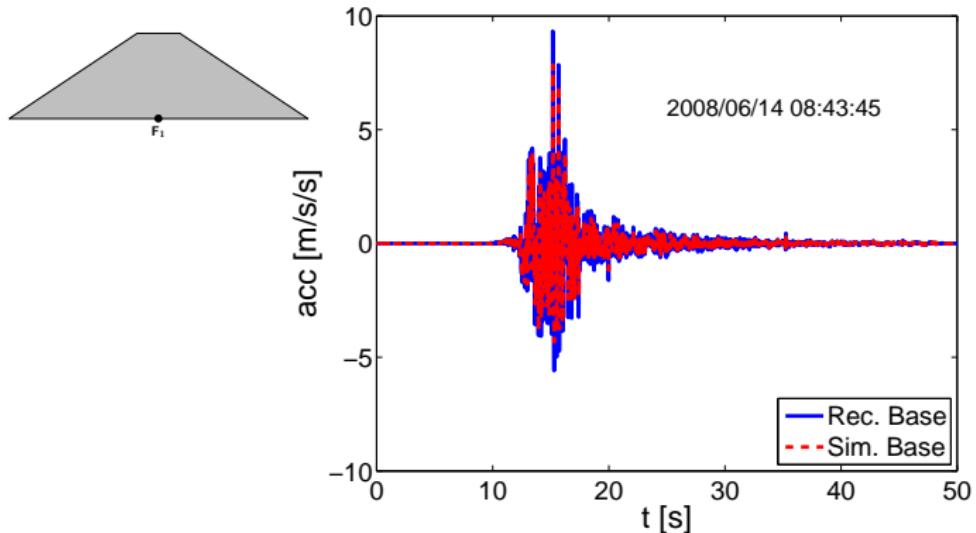
# Numerical model

Southern Akita Pref - 1996



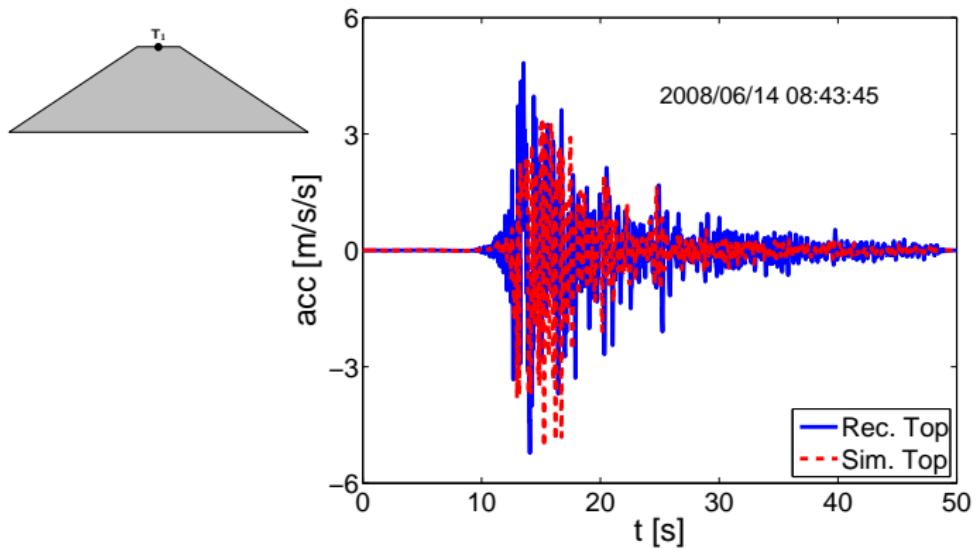
# Numerical model

Southern Iwate Pref - 2008



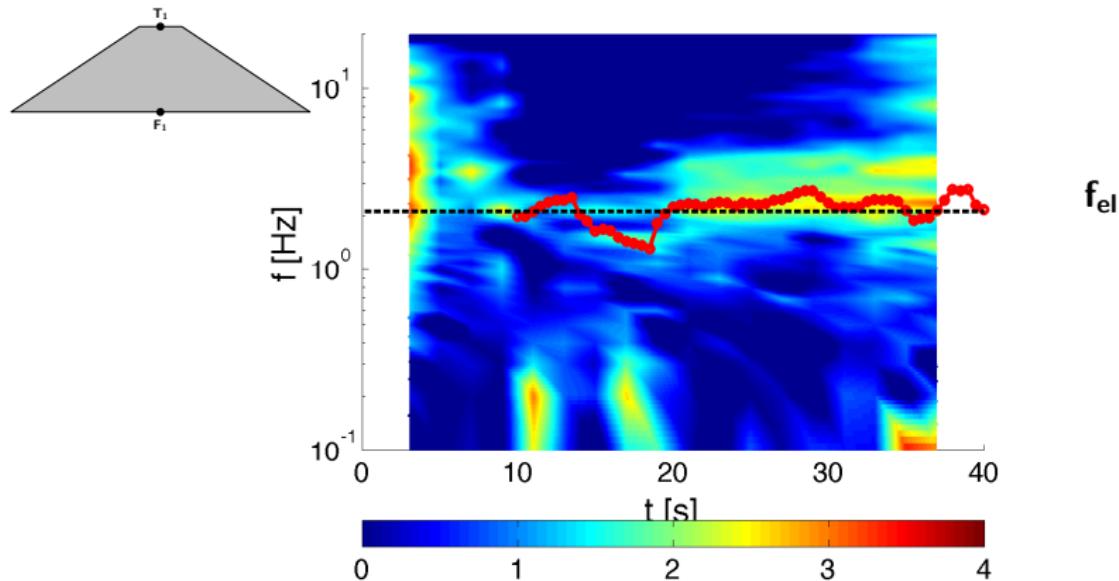
# Numerical model

Southern Iwate Pref - 2008



# Numerical model

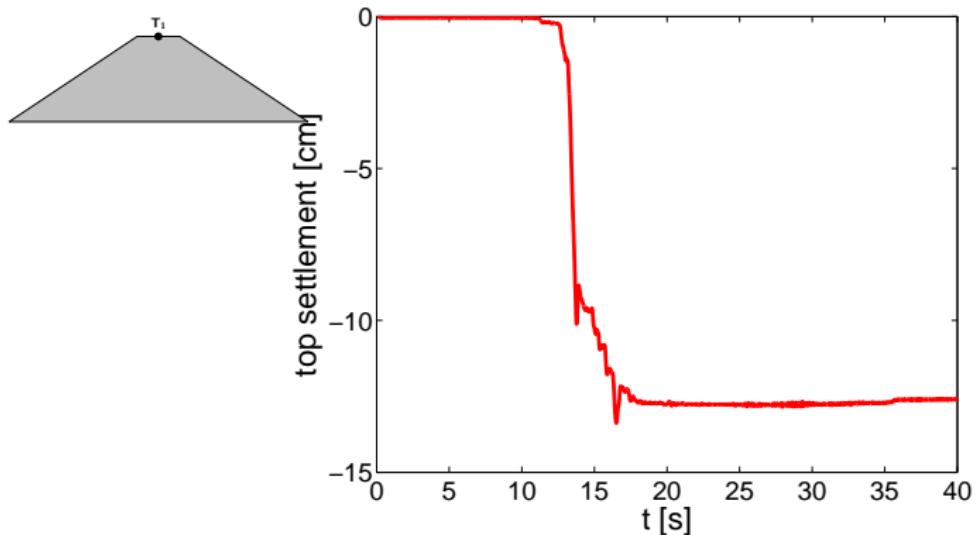
Southern Iwate Pref - 2008



STFT F1-T1- PGA = 1023.8cm/s<sup>2</sup>

# Numerical model

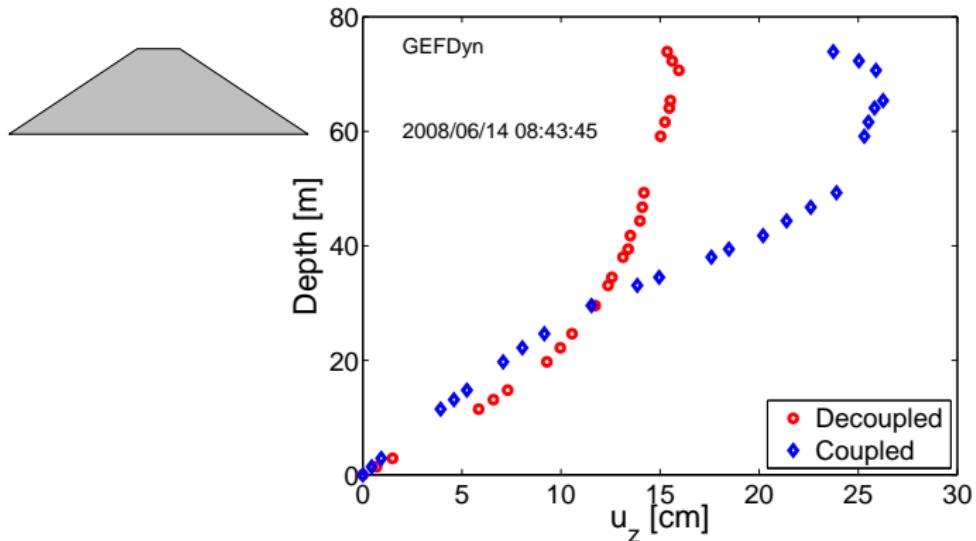
Southern Iwate Pref - 2008



Obtained co-seismic settlement, 13cm - Decoupled behaviour

# Numerical model

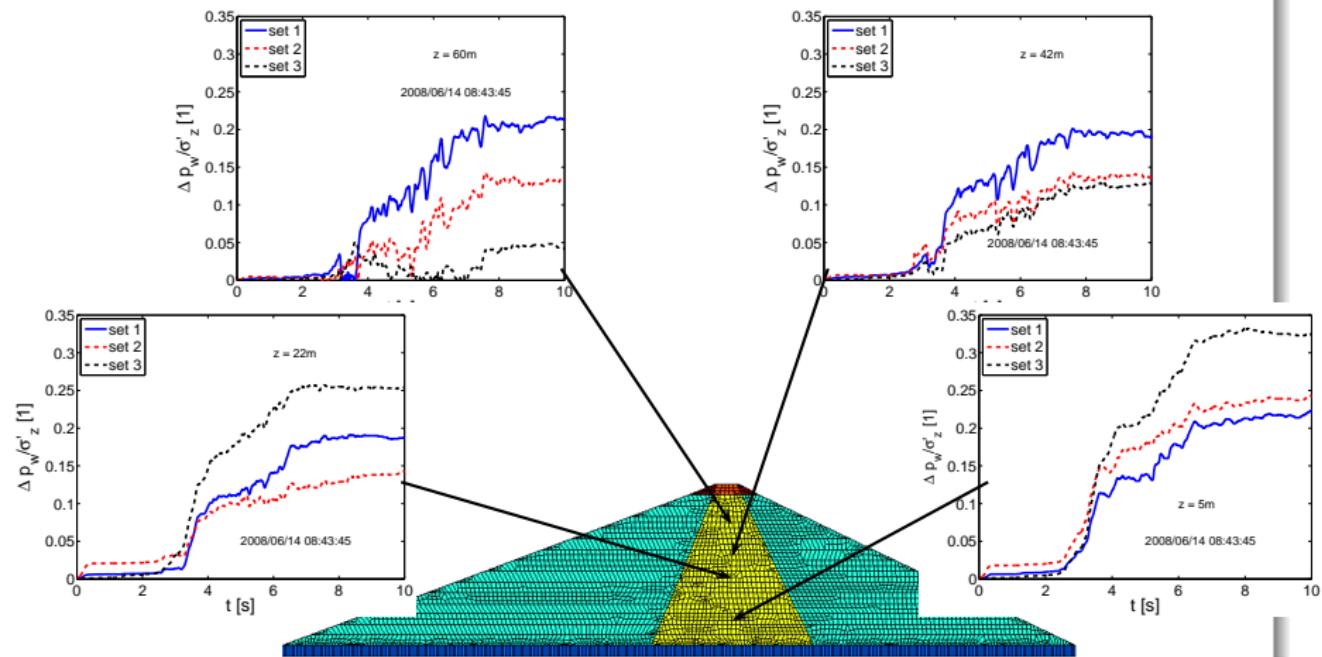
## Southern Iwate Pref - 2008



Obtained co-seismic settlement - *GEFDyn*

# Numerical model

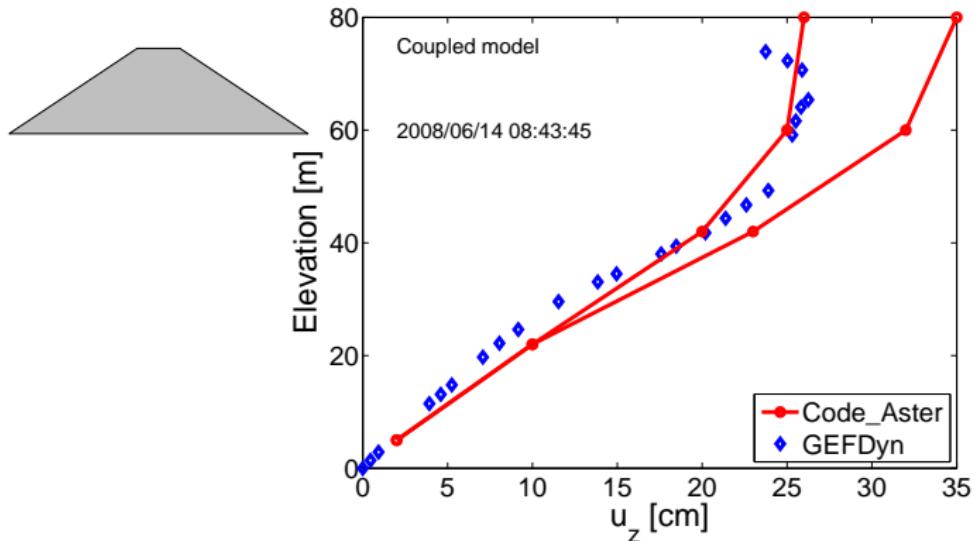
## Southern Iwate Pref - 2008



Obtained  $\Delta p_w / \sigma'_z$ - Coupled behaviour - Code\_Aster

# Numerical model

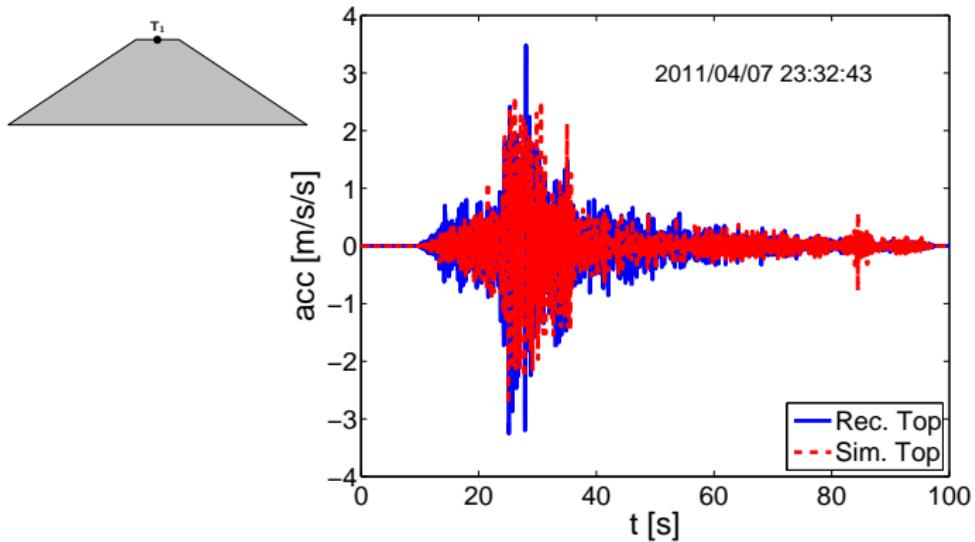
## Southern Iwate Pref - 2008



Obtained co-seismic settlement

# Numerical model

E Off Miyagi Pref - 2011



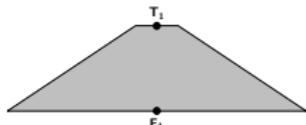
# Anderson Criteria

## Goodness of fit criteria and Frequency Bands

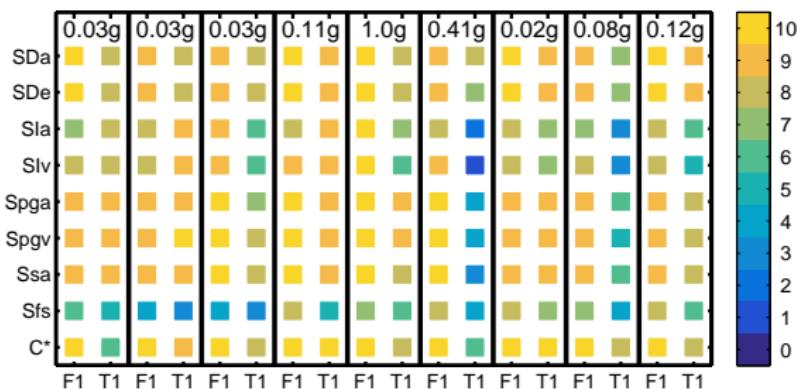
Number	Symbol	Similarity of:	Band	Frequency limits [Hz]
C1	SDa	Arias duration	B1	0.05 - 0.1
C2	SDe	Energy duration	B2	0.1 - 0.2
C3	Sla	Arias Intensity	B3	0.2 - 0.5
C4	Slv	Energy Integral	B4	0.5 - 1.0
C5	Spga	Peak Acceleration	B5	1.0 - 2.0
C6	Spgv	Peak Velocity	B6	2.0 - 5.0
C7	Spgd	Peak Displacement	B7	5.0 - 15.0
C8	Ssa	Response Spectra	B8	0.05 - 15.0
C9	Sfs	Fourier Spectra		
C10	C*	Cross Correlation		

$$C_i(p_1, p_2) = 10 \exp \left\{ - \left[ \frac{(p_1 - p_2)}{\min(p_1, p_2)} \right]^2 \right\} \quad S = \frac{1}{8} \sum_{B=1}^8 \left( \frac{1}{10} \sum_{i=1}^{10} C_{i,B} \right)$$

# Numerical model



## Anderson criteria



# Outline

Recorded signals

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## Conclusions

# Conclusions

- ▶ Used non-linear soil behaviour model is able to represent accurately the recorded behaviour of the dam in the large range of accelerations and frequencies.
- ▶ “*Half-space bedrock's boundary condition*” allows to simulate the borehole condition found at the gallery level.
- ▶ The condition assumed to define the initial state of all materials could be used as a first approach to simulate the dam behaviour.
- ▶ The non-linear behaviour of the dam is concentrated principally at the base of the core material.

A wide-angle photograph of a coastal town at sunset. The sky is filled with warm, orange and pink hues. In the foreground, a sandy beach meets the ocean, with small waves crashing onto a concrete pier. To the right, a long pier extends into the water, lined with a blue metal railing. In the background, a dense cluster of buildings with dark roofs and light-colored facades is built along the coastline. The overall atmosphere is peaceful and scenic.

Thank you for your attention  
Dōmo arigatō gozaimas[u]

-  Aubry, D., Hujeux, J.-C., Lassoudière, F., and Meimon, Y. (1982).  
A double memory model with multiple mechanisms for cyclic soil behaviour.  
In *International symposium on numerical models in geomechanics*, pages 3–13. Balkema.
-  Hujeux, J.-C. (1985).  
Une loi de comportement pour le chargement cyclique des sols.  
In *Génie Parasismique*, pages 278–302. V. Davidovici, Presses ENPC, France.
-  Lopez-Caballero, F., Modaressi, A., and Elmi, F. (2003).  
Identification of an elastoplastic model parameters using laboratory and in-situ tests.  
In *Deformation Characteristics of Geomaterials*, pages 1183–1190. Eds. H. Di Benedetto et al., A.A. Balkema, ISBN 9058096041.
-  Montoya-Noguera, S. (2016).  
*Assessment and mitigation of liquefaction seismic risk: Numerical modeling of their effects on SSI*.  
PhD thesis, École CentraleSupélec, France.
-  Montoya-Noguera, S. and Lopez-Caballero, F. (2016).  
Effect of coupling excess pore pressure and deformation on nonlinear seismic soil response.  
*Acta Geotechnica*, 11(1):191–207.
-  Ohmachi, T. and Taharz, T. (2011).  
Nonlinear earthquake response characteristics of a central clay core rockfill dam.  
*Soils and Foundations*, 51(2):227–238.
-  Seed, H. B., Wong, R. T., Idriss, I. M., and Tokimatsu, K. (1986).  
Moduli and damping factors for dynamic analyses of cohesionless soils.  
*Journal of Geotechnical Engineering - ASCE*, 112(11):1016–1032.