



International Symposium
Qualification of dynamic analyses of dams and their equipments
and of probabilistic assessment seismic hazard in Europe
31th August – 2nd September 2016 – Saint-Malo

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Japan Dam Engineering Center

Session : 4

THE REPRODUCTION ANALYSIS OF ARATOZAWA DAM DURING 2008 EARTHQUAKE



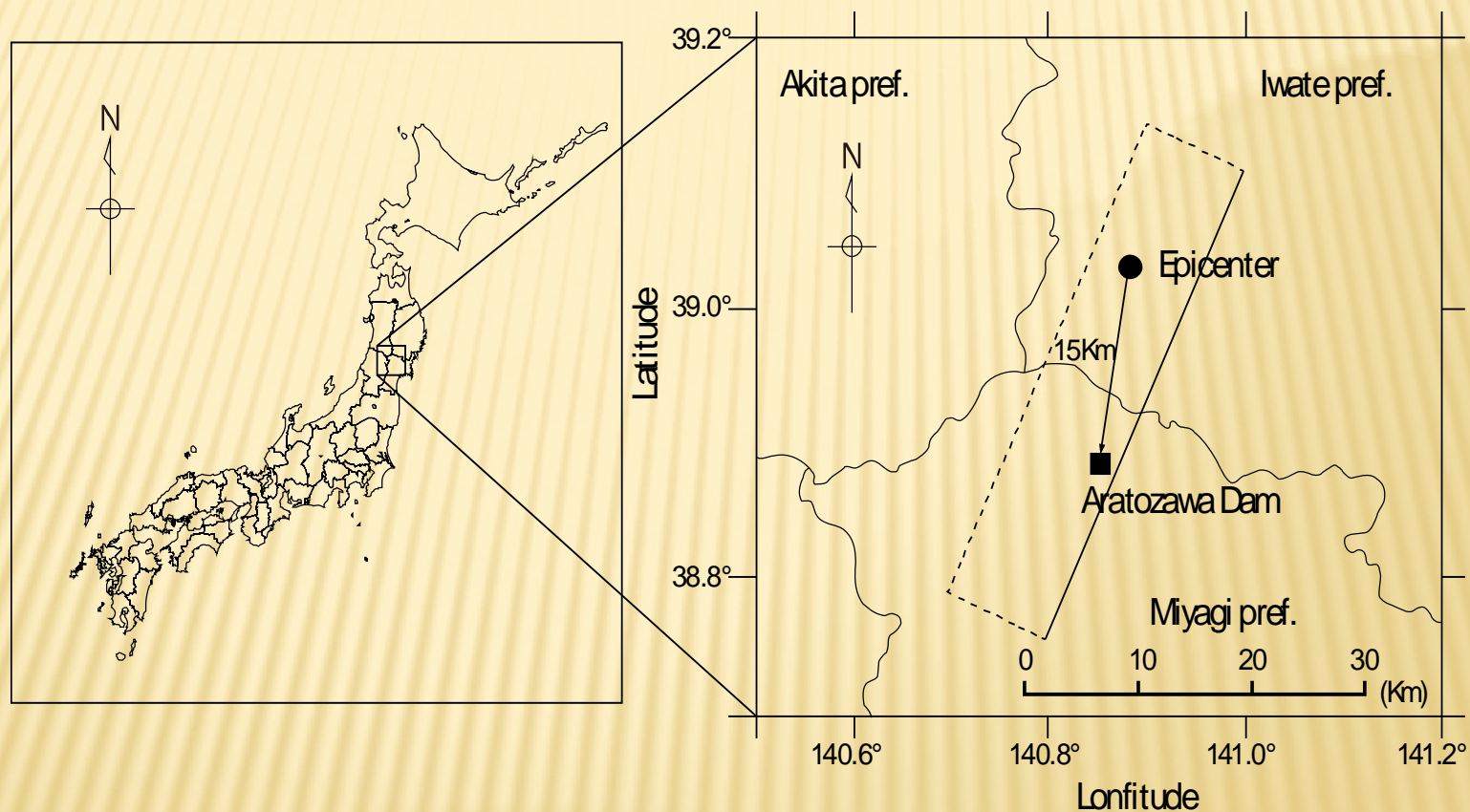
1. Main Features of Aratozawa dam



*Homepage of
Miyagi
Prefectural
Government*

Dam type	Rockfill dam with central clay core (1998)
Dam height	74.4 m
Crest length	413.7 m
Crest width	10.0 m
Slope gradients	Upstream: 1:2.7 , Downstream: 1:2.1
Design seismic coefficient	0.15 (dam body), 0.18 (intake tower, bridge) 0.16 (spillway)

2. Location of Aratozawa dam



3. Gigantic landslide by the Earthquake



after Takashi IGUCHI

20cm of settlement at dam crest

after Kyodo Press

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

4. Repair work of landslide

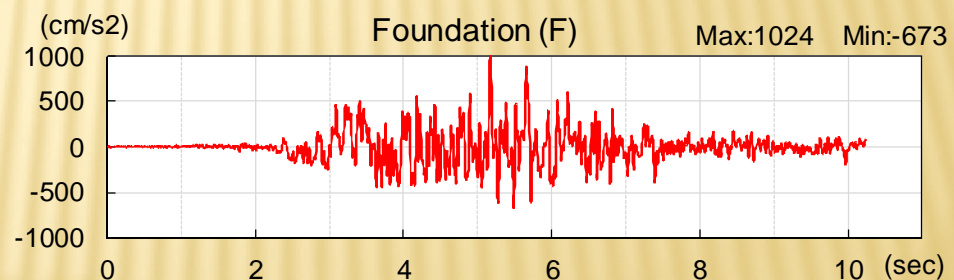
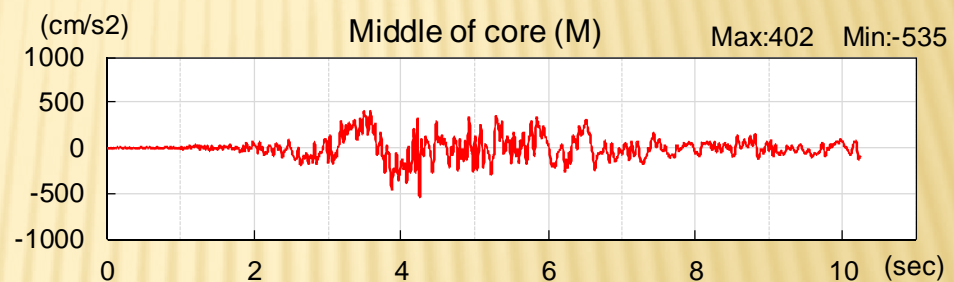
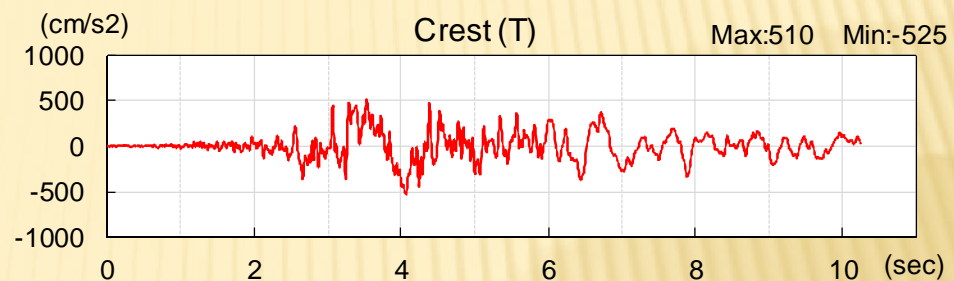
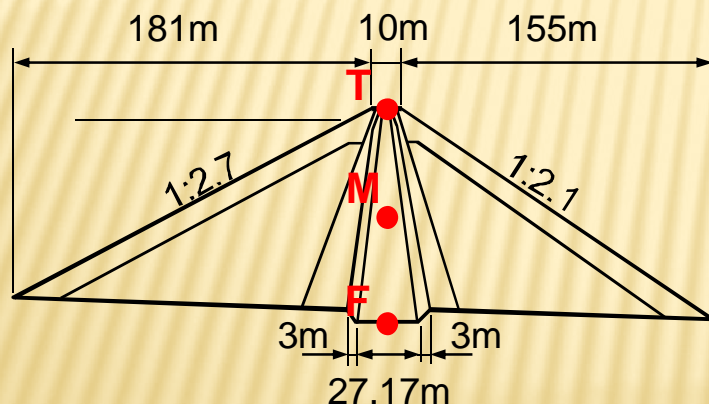
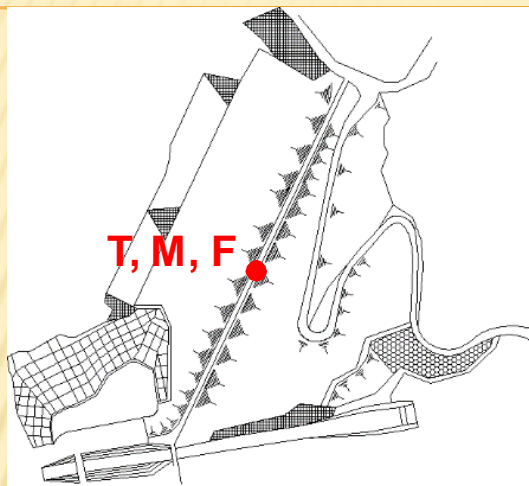


2009/Oct/16



2011/Apr./6

5. Earthquake monitoring (1/2)



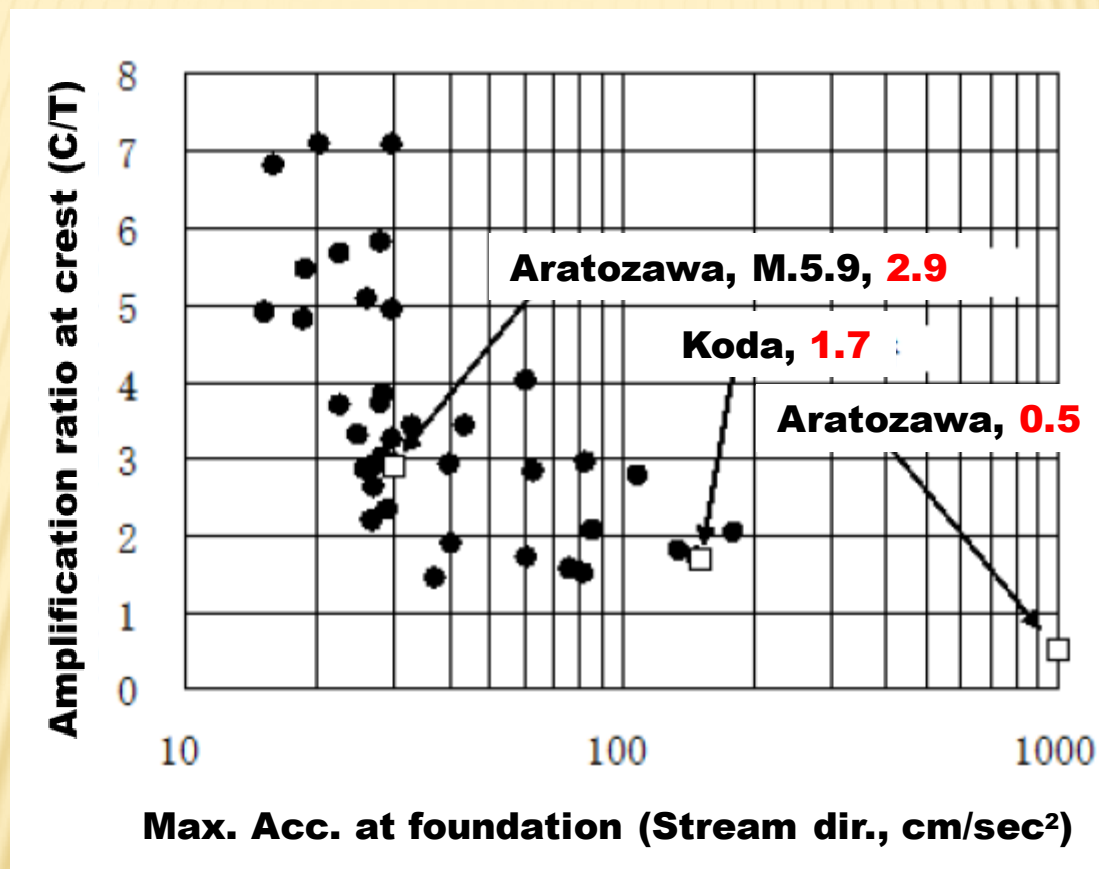
Locations of Seismographs

Earthquake Records in Stream Dir.

Iwate-Miyagi Nairiku Earthquake, 2008

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

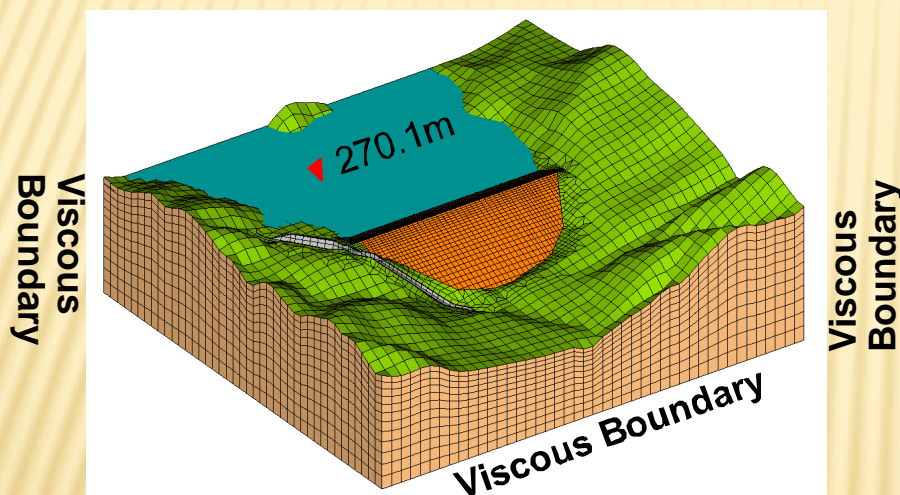
5. Earthquake monitoring (2/2)



Acceleration amplification ratio of dam body (Crest/Foundation)

6. Investigation with numerical analysis

- (1) Why such peculiar phenomenon occurred ?
- (2) What caused the permanent deformation ?



Analytical Model

Method for Earthquake Behavior Simulation
3-D FEM model
Equivalent Linear Analysis

Method for Permanent Deformation Reproduction
a) Stability analysis based on circular slip surface
b) Deformation calculation based on the theory of cumulative damage

6-1 Procedure of numerical analysis

(1) Go Identification



(2) Reproduction of earthquake motion at the bottom of model



(3) Identification of reference strain and damping ratio

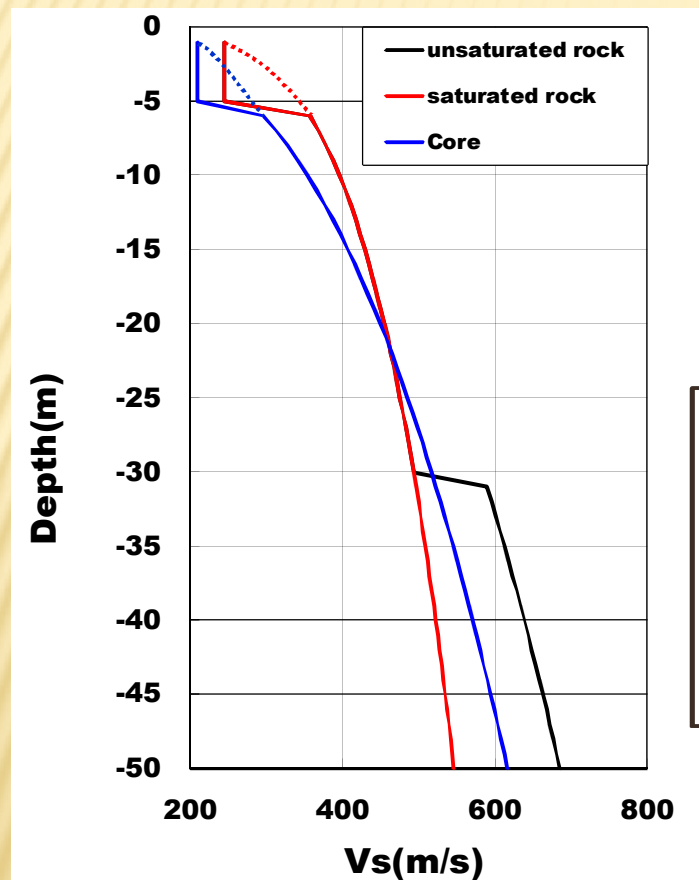


(4) Simulation of the dynamic behavior in Iwate-Miyagi Nairiku earthquake



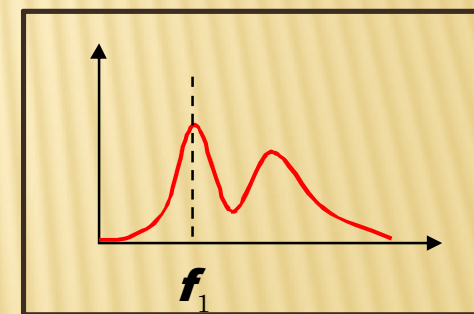
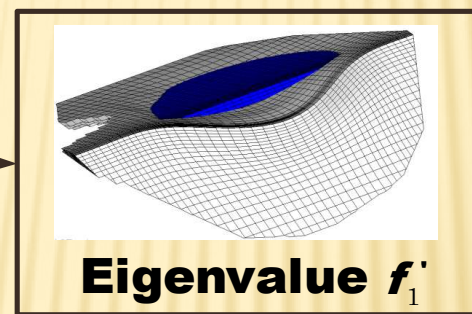
(5) Calculation of permanent deformation

6-2 Identification of initial shear moduli G_0

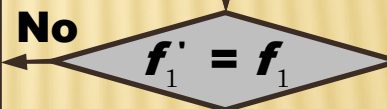


after Sawada and Takahashi (1975)

$$G = \frac{\rho}{g} V_s^2$$



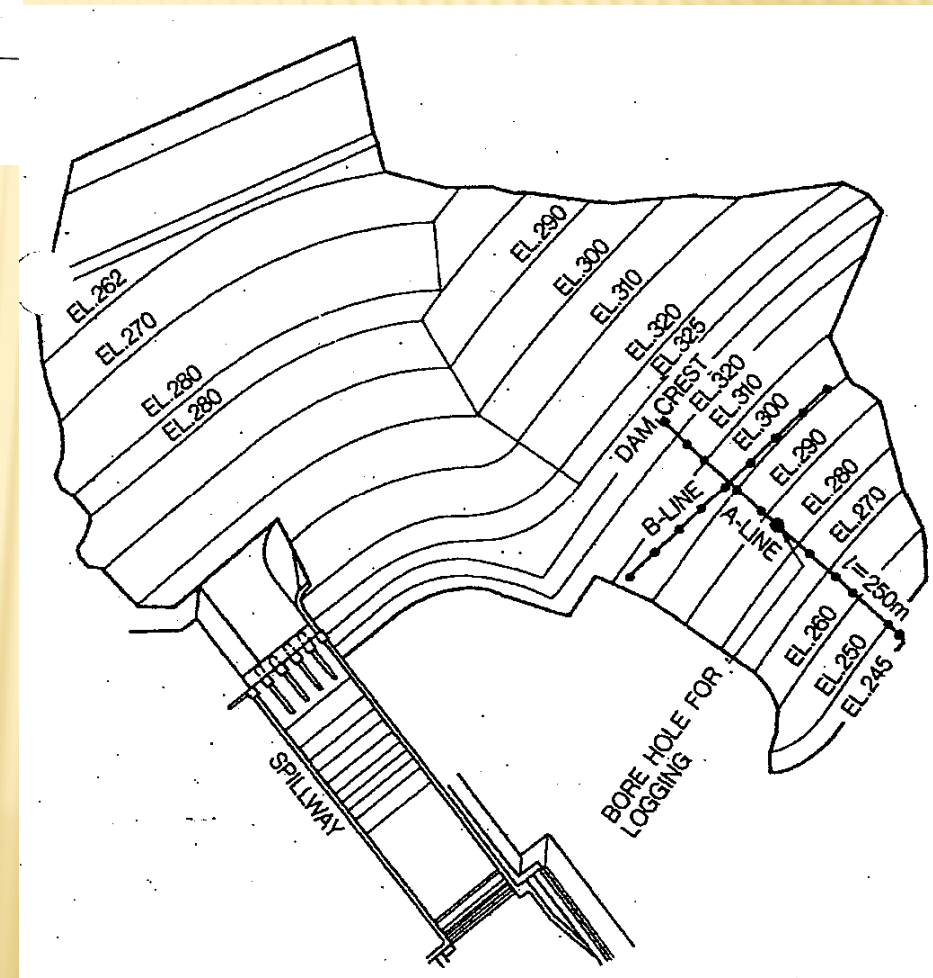
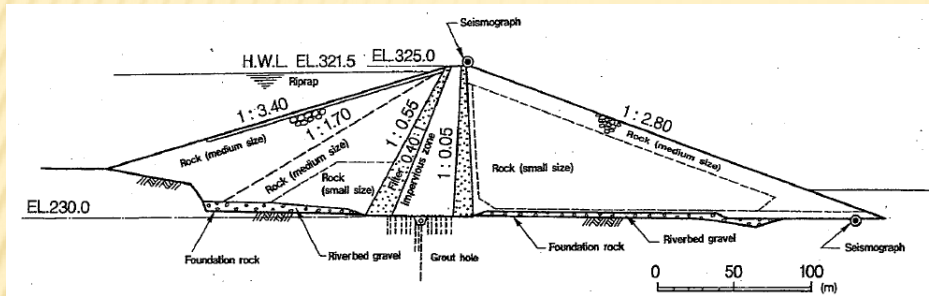
By adjusting G



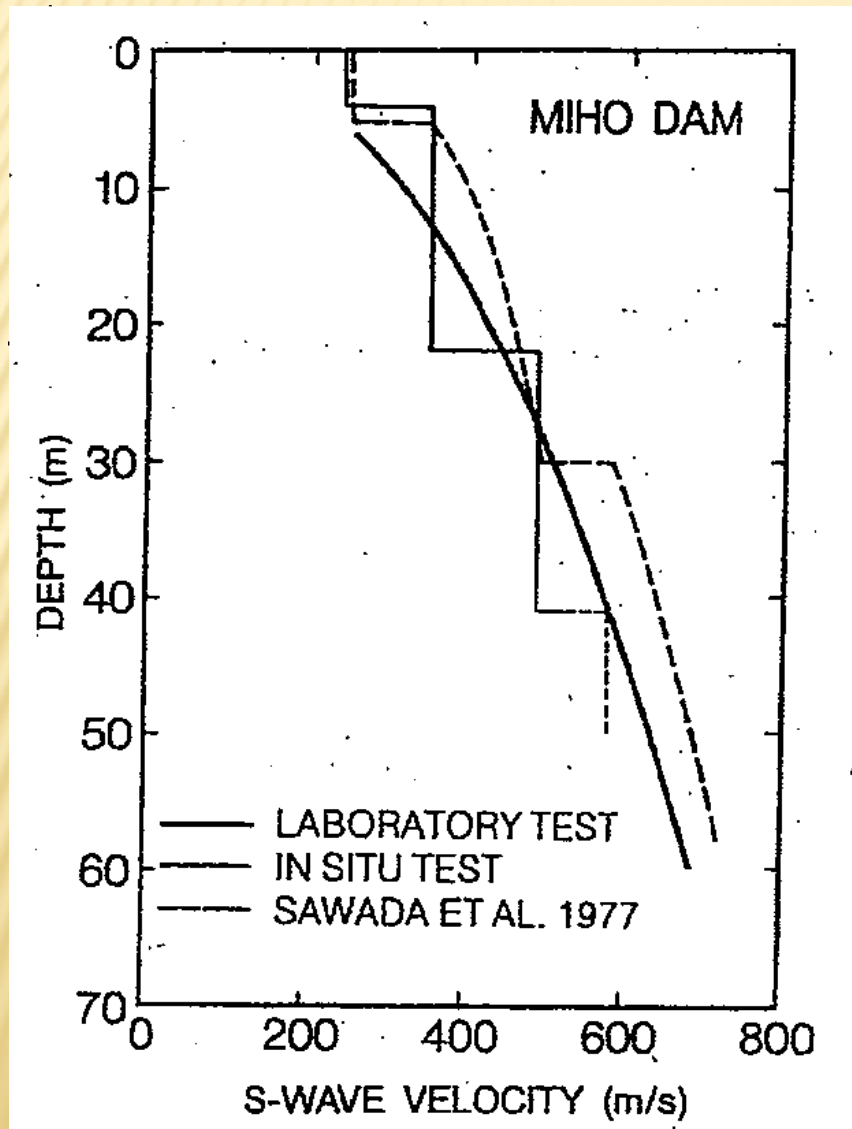
G_0

Measurement of shear wave velocity

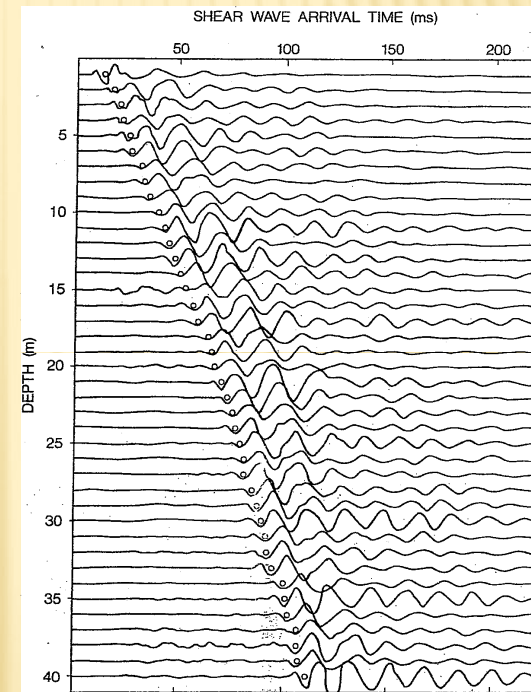
Case of Miho dam



Measurement of shear wave velocity



Case of Miho dam



$$G_{\max} = 558 P_a \frac{(2.17 - e)^2}{1 + e} \left(\frac{\sigma'_m}{P_a} \right)^{0.88} \quad (\text{Miho Dam})$$

$$\sigma'_m = \frac{1}{3} (1 + K) \rho_t g D$$

$$V_s = \left(\frac{G_{\max}}{\rho_t} \right)^{0.5}$$

Measurement of shear wave velocity

Case of Miho dam

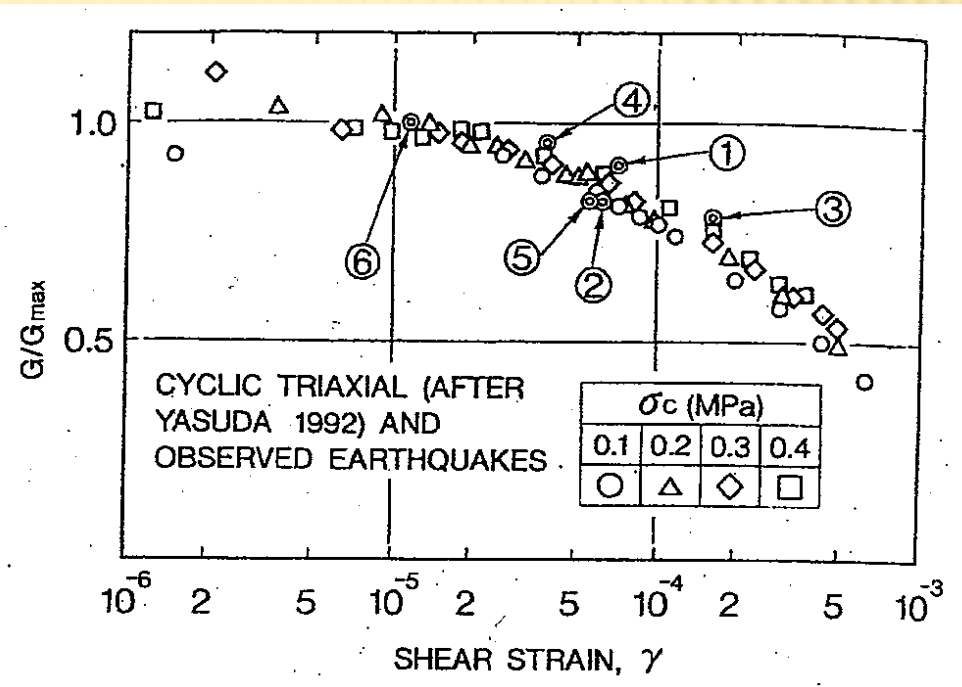
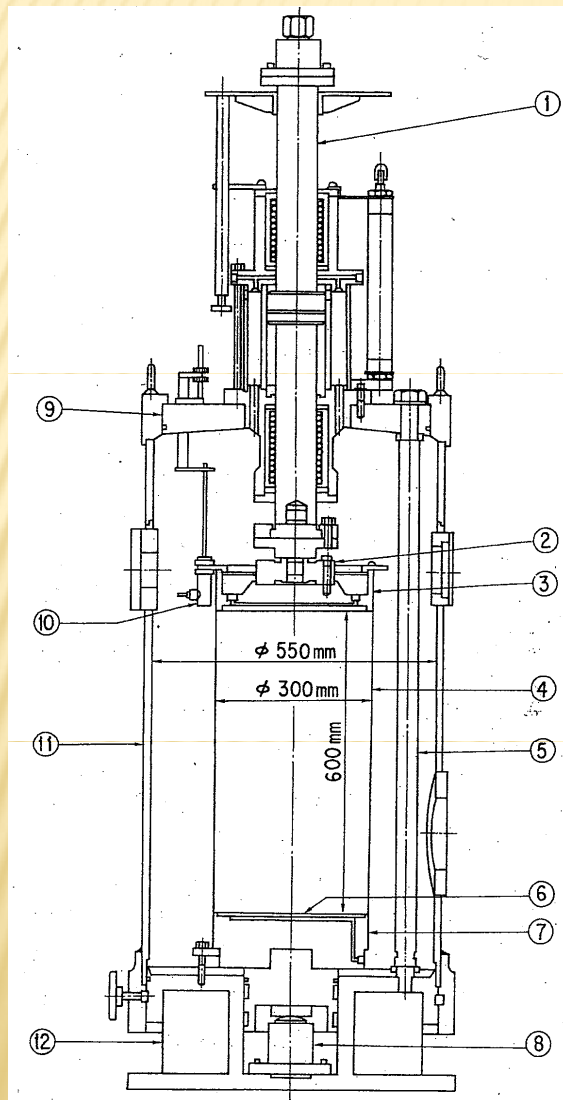
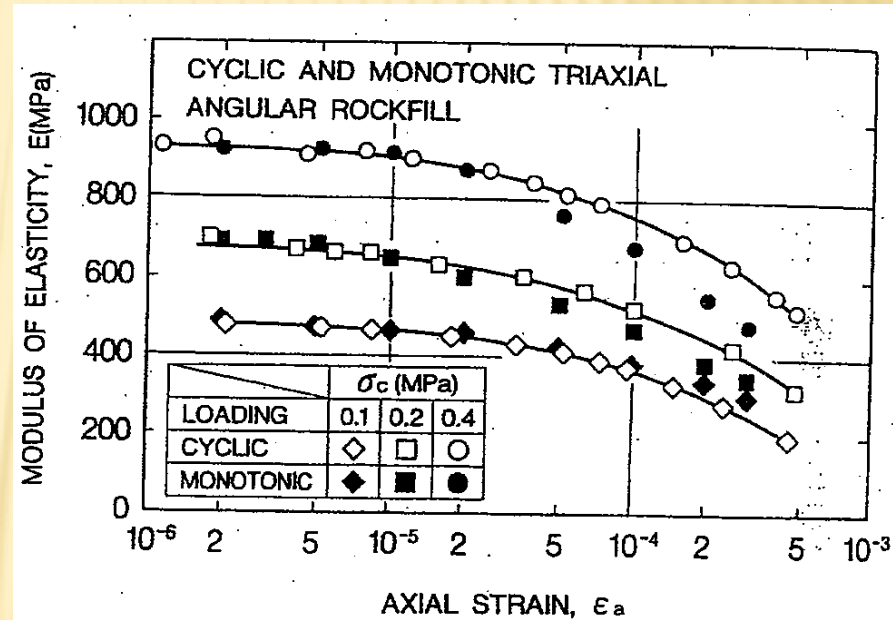
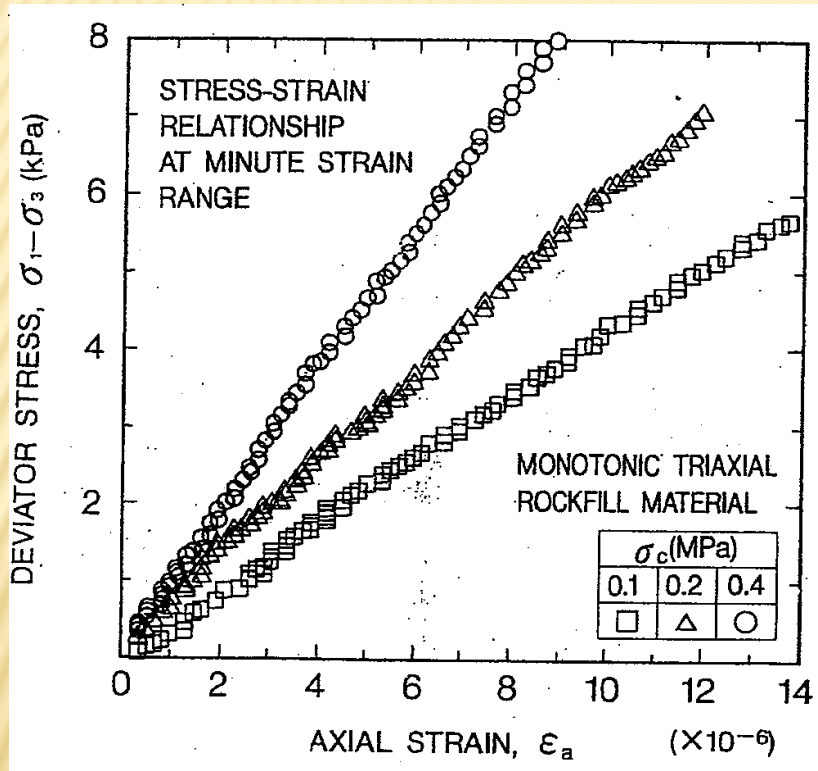


TABLE 4. Data from six earthquakes at Miho Dam for comparison with those from laboratory

Name of earthquake	Date	Magnitude	1st natural frequency	Shear strain	Reduction of shear modulus
1 Izu Peninsula Off	29 June 1980	6.7	1.95	6.37×10^{-5}	0.91
2 East Yamanashi	14 April 1981	4.5	1.86	5.40×10^{-5}	0.82
3 West Kanagawa	8 Aug. 1983	6.0	1.81	1.63×10^{-4}	0.78
4 Chiba West Off	17 Dec. 1987	6.7	2.00	4.00×10^{-5}	0.95
5 Hakone	5 Aug. 1990	5.1	1.86	5.35×10^{-5}	0.82
6 Tokyo Bay	2 Feb. 1992	5.7	2.05	2.22×10^{-5}	1.00

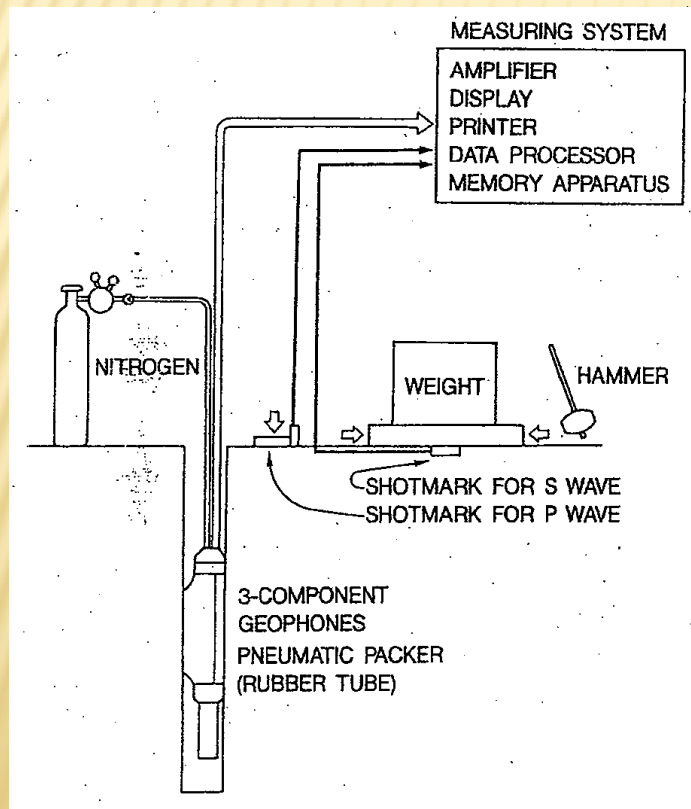
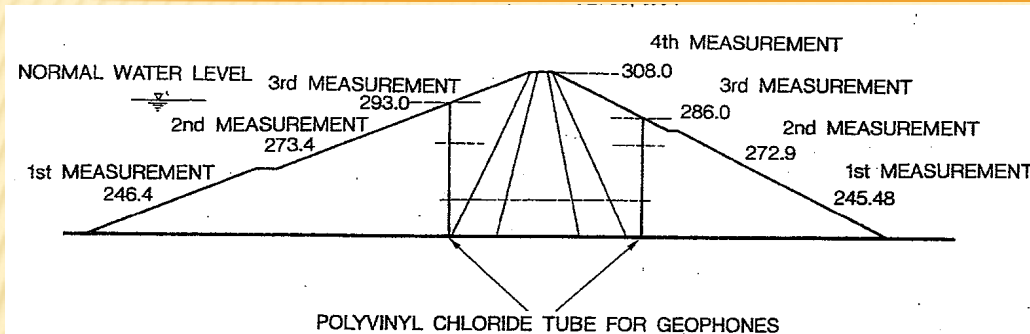
Measurement of shear wave velocity

Case of Miho dam



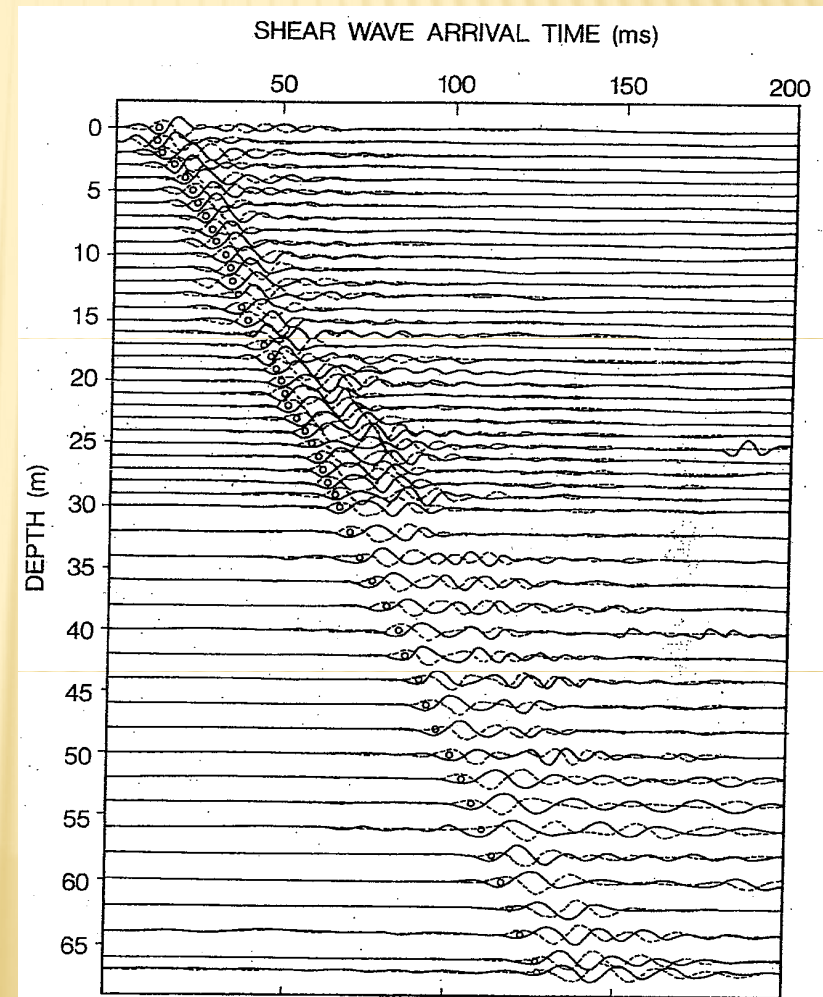
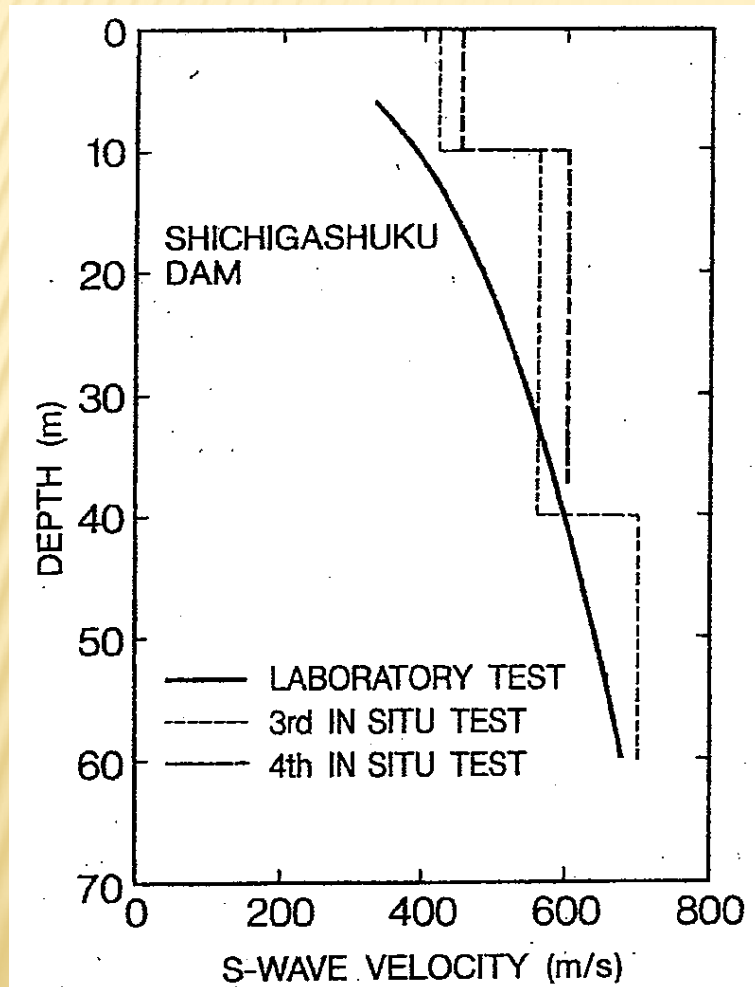
Measurement of shear wave velocity

Case of Shichigasyuku dam

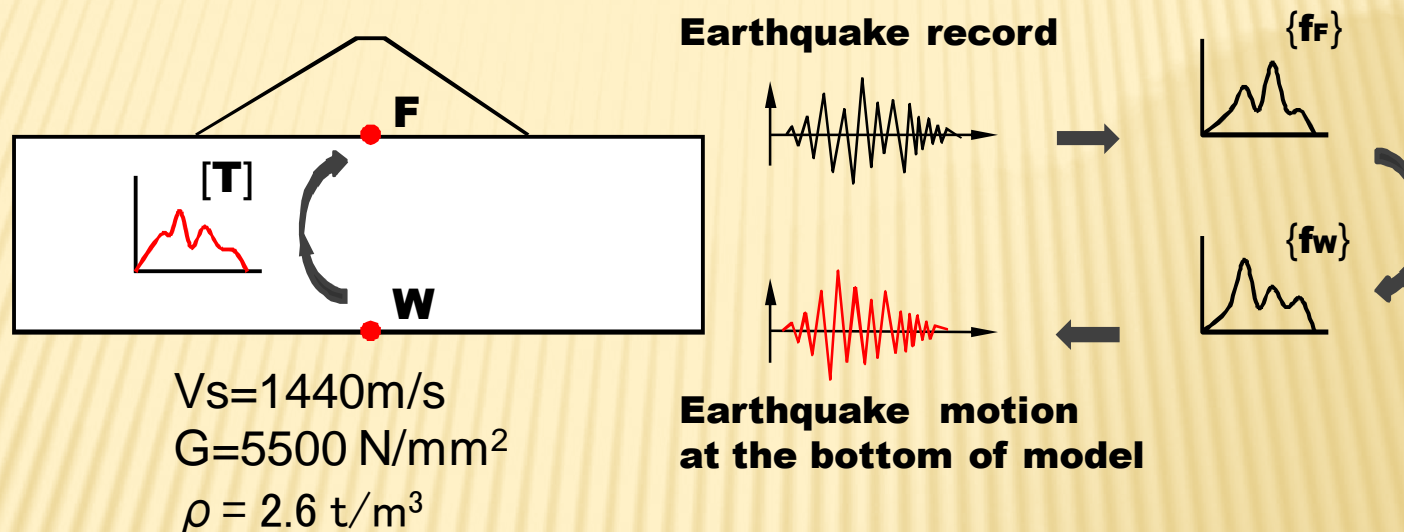


Measurement of shear wave velocity

Case of Shichigasyuku dam



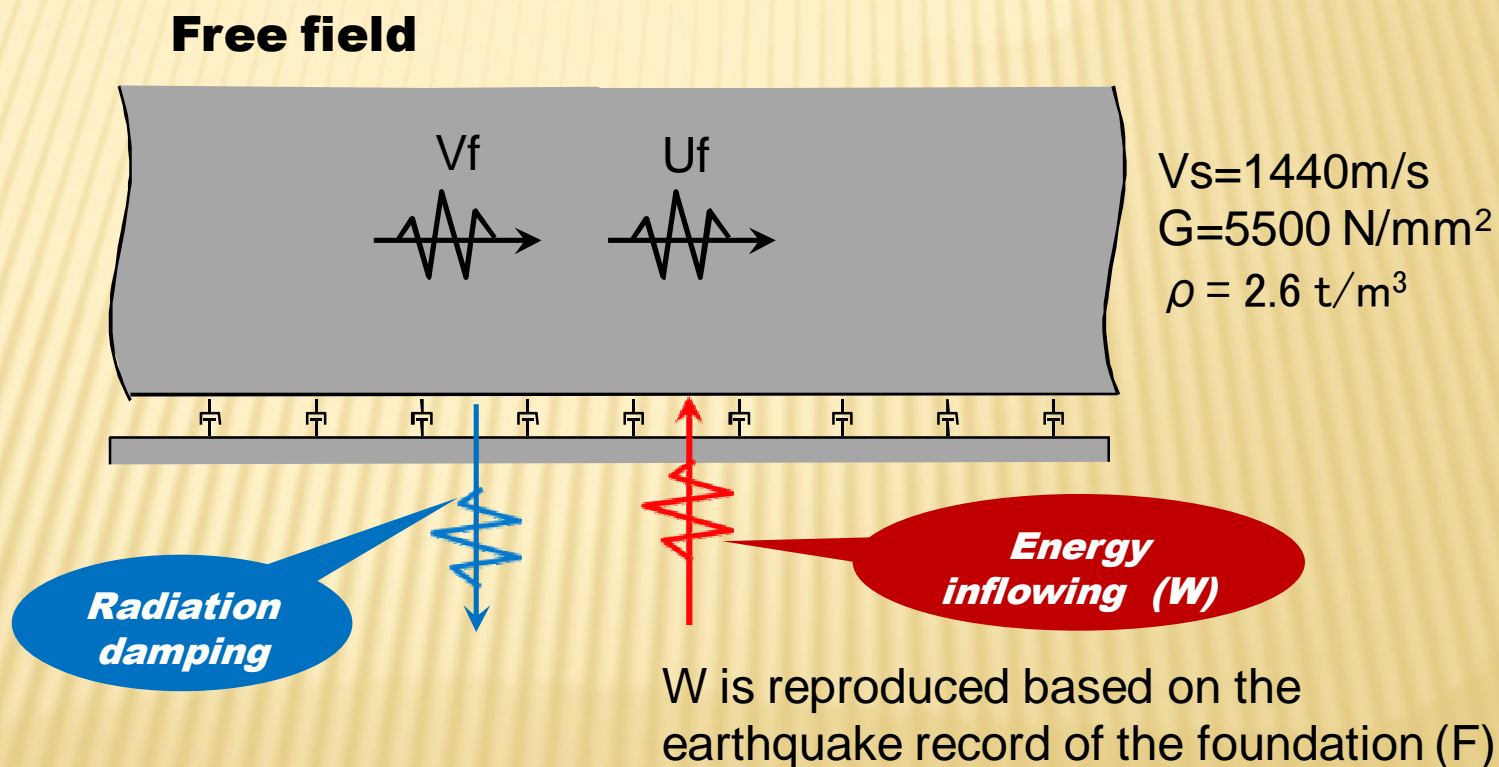
6-3 Reproduction of Earthquake Motion at the Bottom of the Analytical Model



$$\begin{bmatrix} F_{AX} \\ F_{AY} \\ F_{AZ} \end{bmatrix} = \begin{bmatrix} T_{XX} & T_{YX} & T_{ZX} \\ T_{XY} & T_{YY} & T_{ZY} \\ T_{XZ} & T_{YZ} & T_{ZZ} \end{bmatrix} \begin{bmatrix} F_{BX} \\ F_{BY} \\ F_{BZ} \end{bmatrix}$$

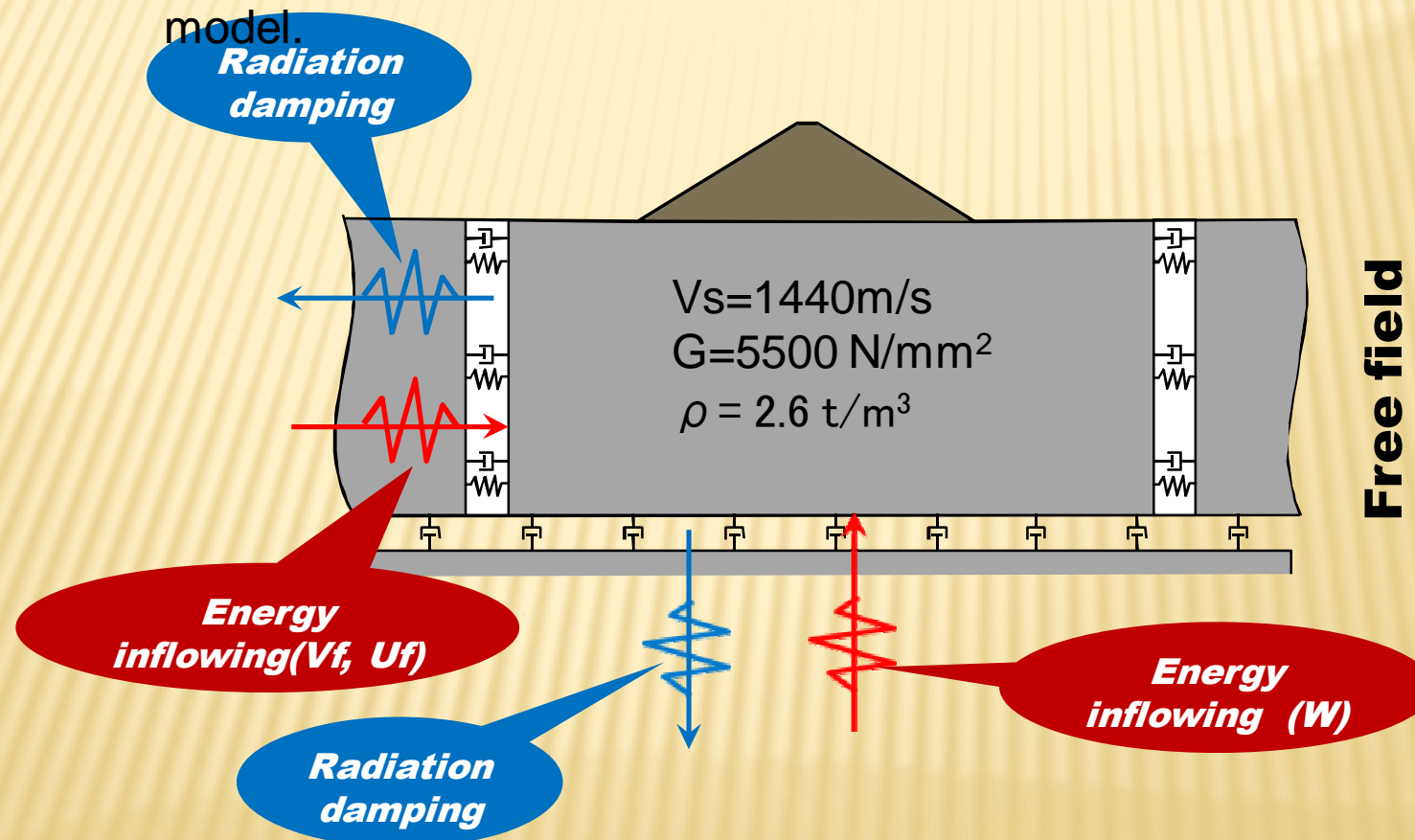
6-4 Boundary condition (1/2)

First, input the earthquake wave (W) to the bottom of free field, to get the velocity response (V_f) and the displacement response (U_f) of the free field.

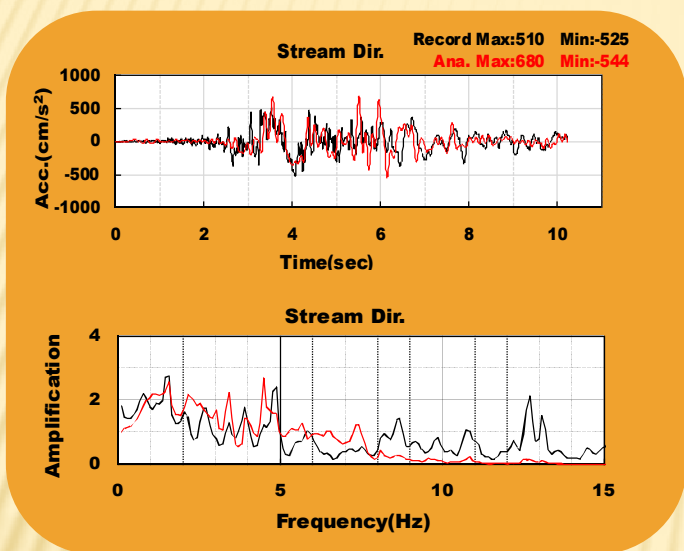


6-4 Boundary condition (2/2)

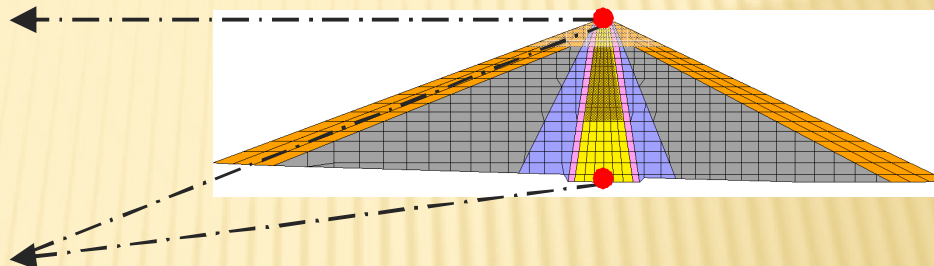
Then, input the earthquake wave (W) to the bottom of foundation, and at the same time, input the responses of the free field (V_f , U_f) to the lateral boundary of the foundation, to get the responses of the whole model.



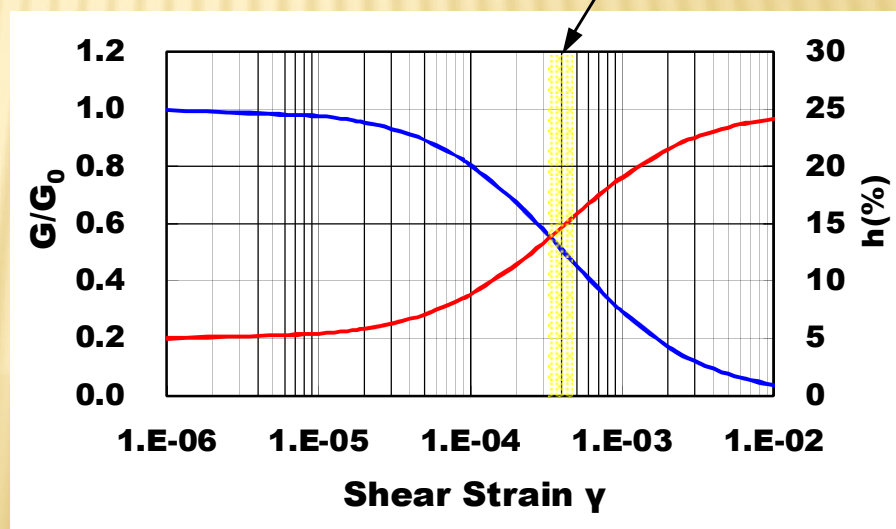
6-5 Identification of reference strain and damping ratio (1/2)



Earthquake Response Analysis



Reference strain γ_r



By adjusting γ_r & h_{max}

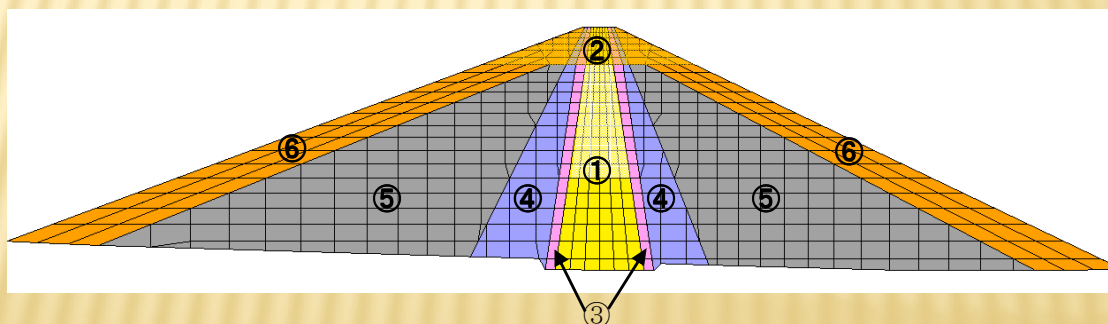
$$\frac{G}{G_0} = \frac{1}{1 + \gamma/\gamma_r}$$

$$h = h_{max} \frac{\gamma}{\gamma + \gamma_r} + h_0$$

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

6-5 Identification of reference strain and damping ratio (2/2)

Category	Max. damping	Reference shear strain
① Core (lower parts)	20%	3.0×10^{-4}
② Core (upper part)	30%	
③ Filter ④ Transition	30%	4.0×10^{-4}
⑤ Rock (inner)	23%	
⑥ Rock (outer)		



Identification of material properties

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

6-6 Damping and it's nonlinearity

Rayleigh type

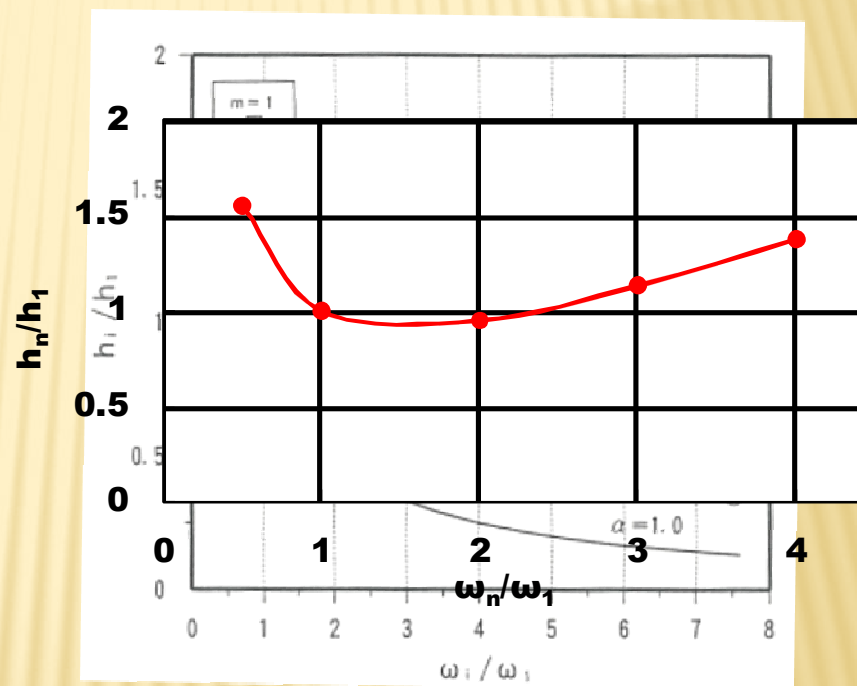
$$[C]^e = \alpha[M]^e + \beta[K]^e$$

Here:

$$\alpha = f(h, \omega_1)$$

$$\beta = g(h, \omega_1)$$

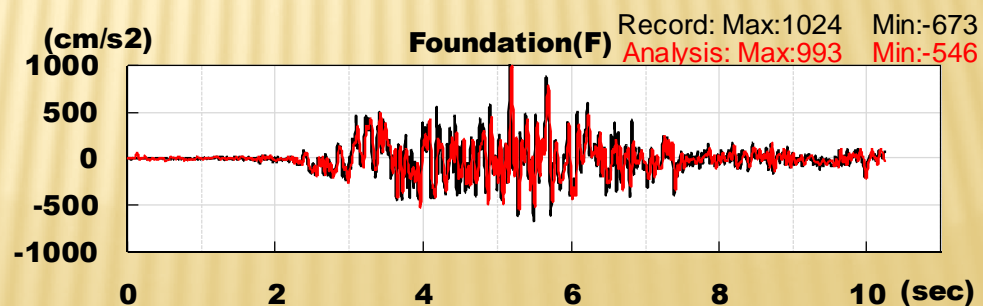
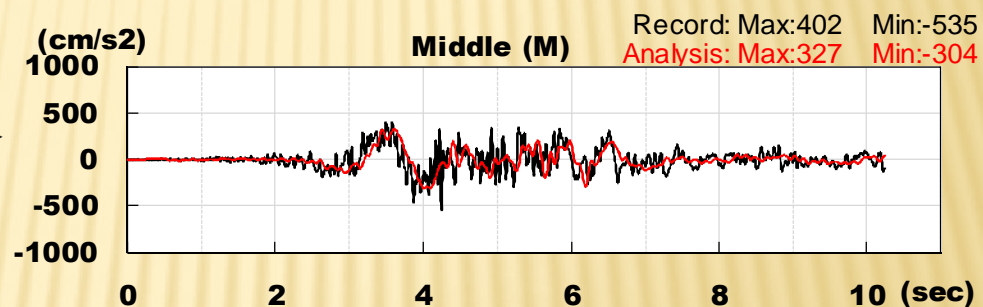
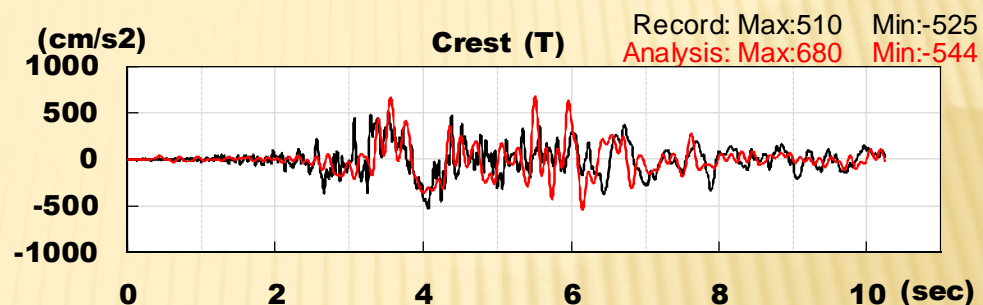
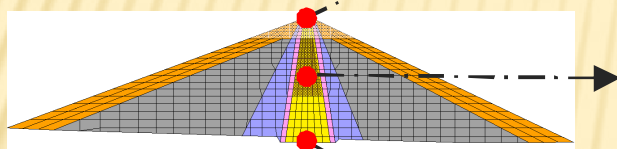
$$h = h_{\max} \frac{Y}{Y + Y_r} + h_0$$



Based on the frequency independency of internal damping of soil materials

7-1 Acceleration response (1/3)

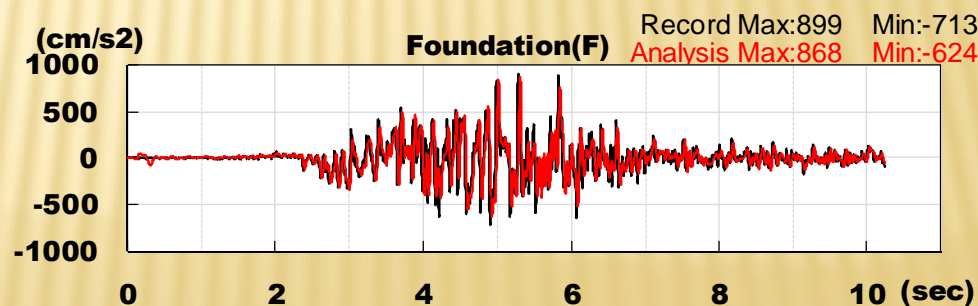
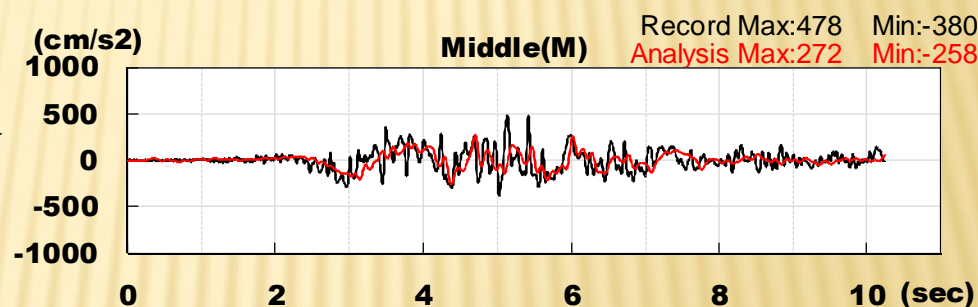
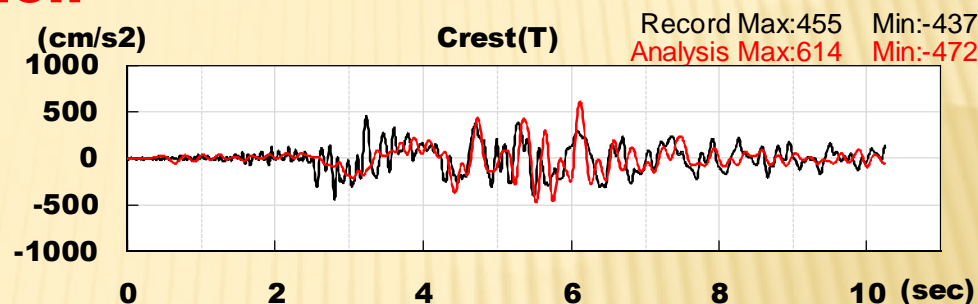
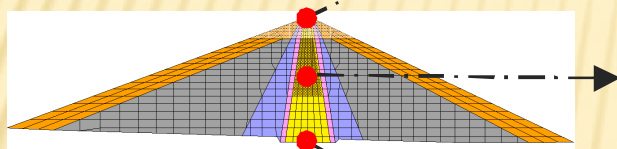
Stream Direction



— **Record**
— **Analysis**

7-1 Acceleration response (2/3)

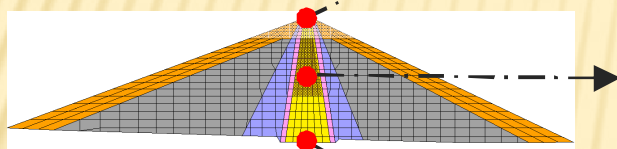
Cross Stream Direction



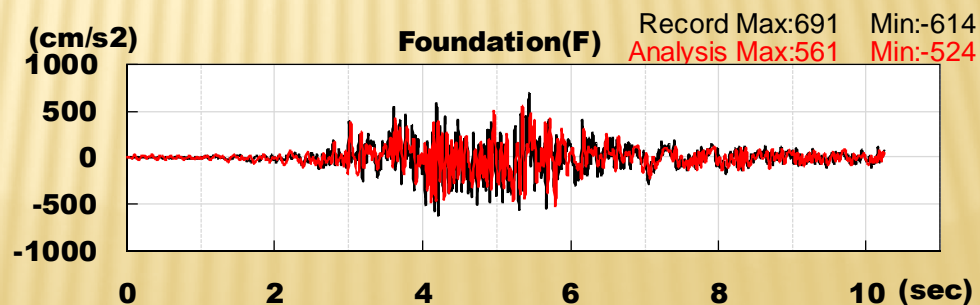
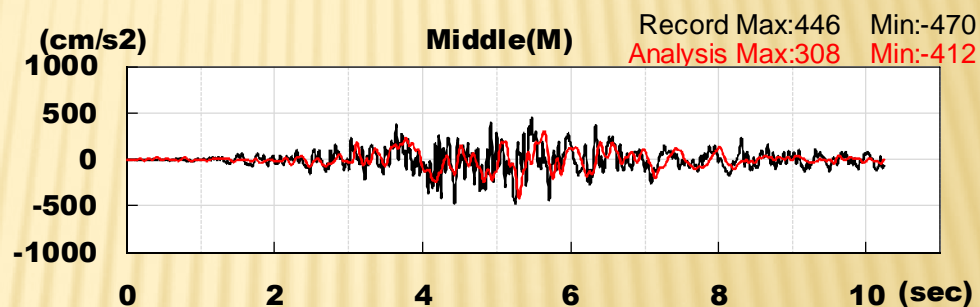
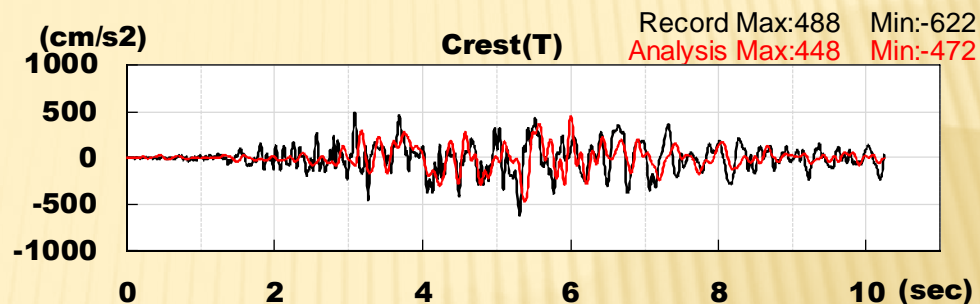
— **Record**
— **Analysis**

7-1 Acceleration response (3/3)

Vertical Direction

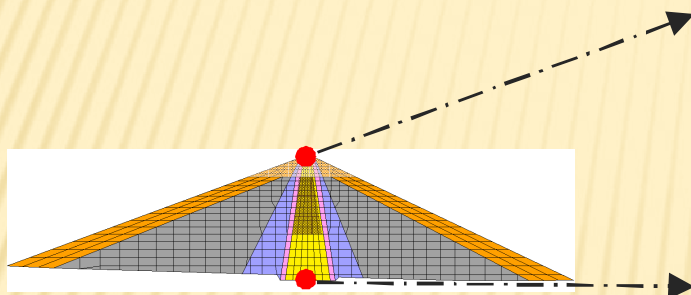


— **Record**
— **Analysis**

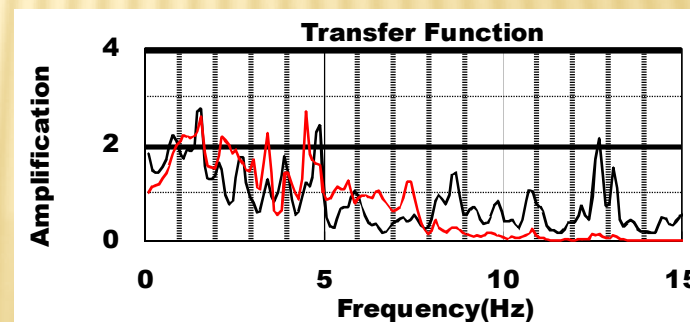
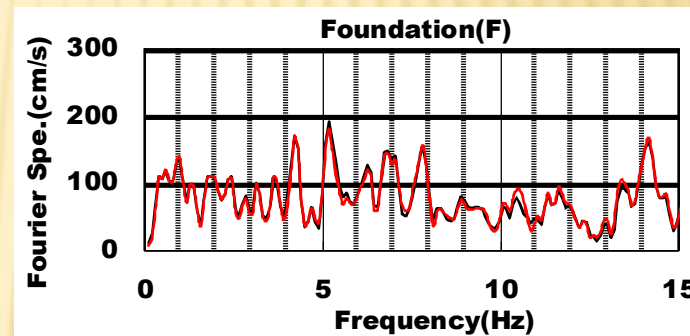
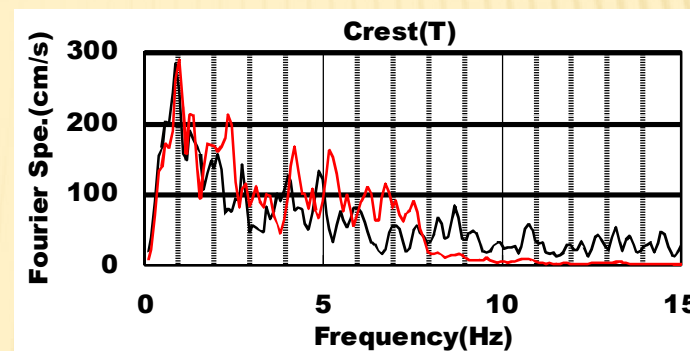


7-2 Spectra & transfer functions (1/6)

Stream Direction

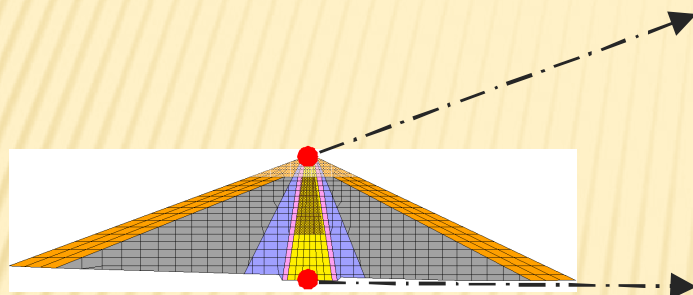


— **Record**
 — **Analysis**

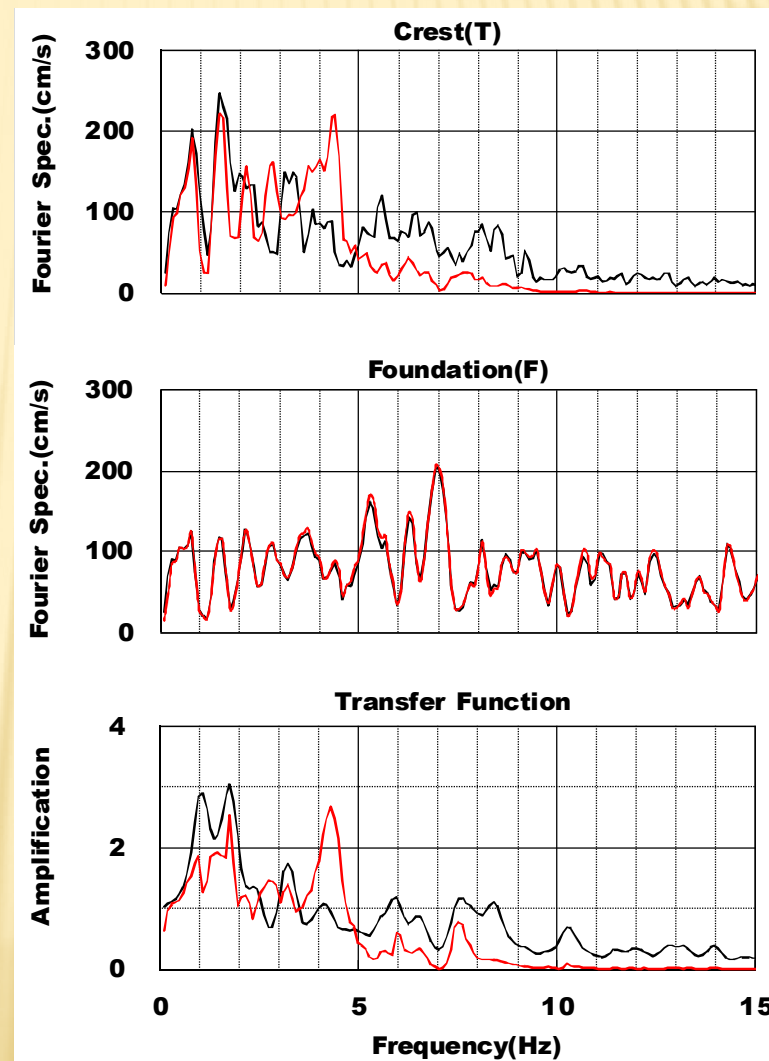


7-2 Spectra & transfer functions (2/6)

Cross Stream Direction

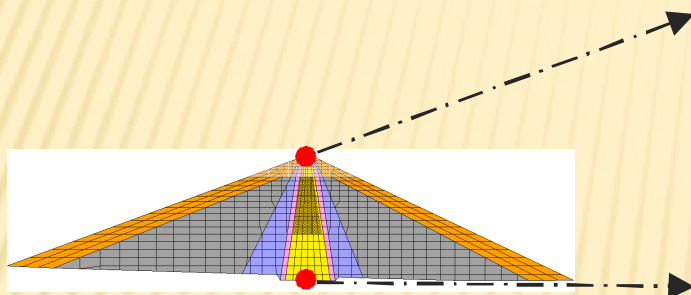


— **Record**
 — **Analysis**

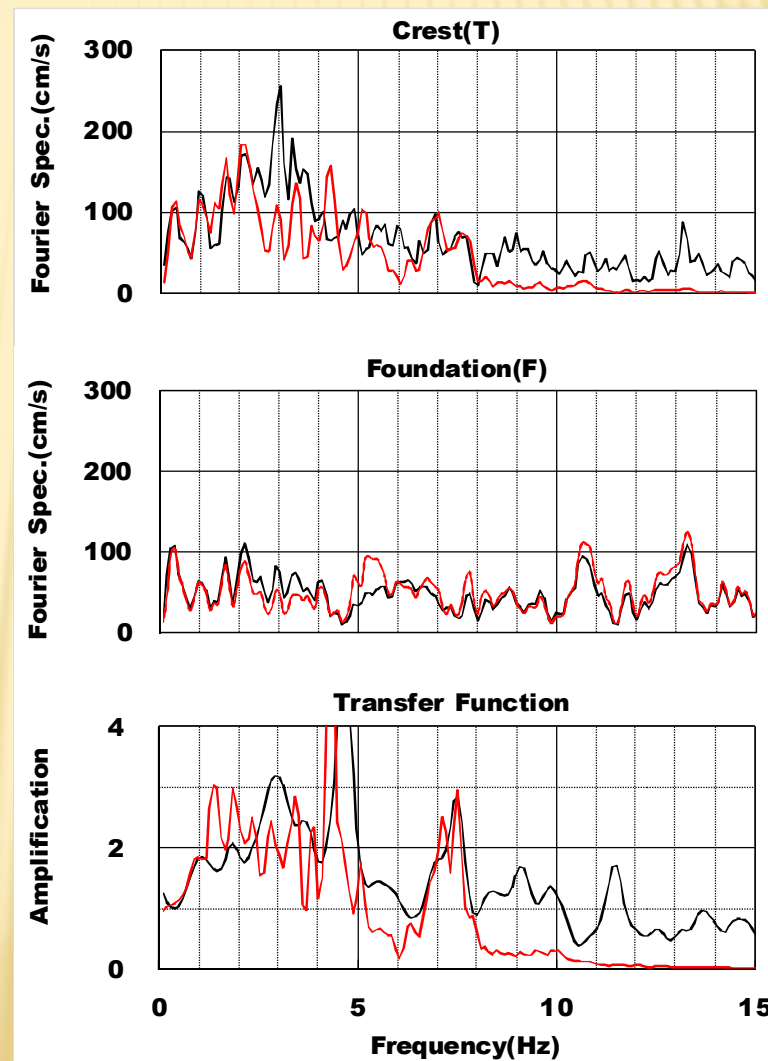


7-2 Spectra & transfer functions (3/6)

Vertical Direction

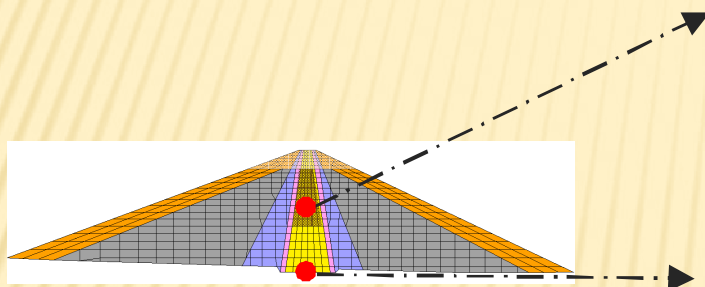


— **Record**
 — **Analysis**

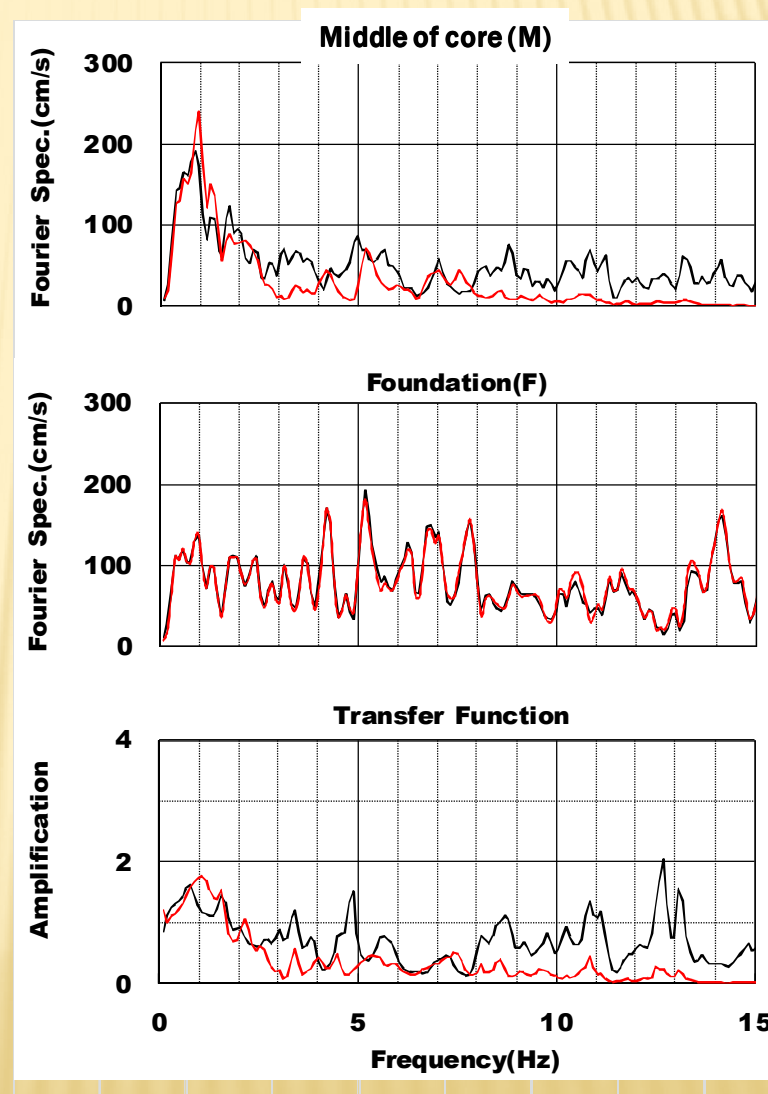


7-3 Spectra & transfer functions (4/6)

Stream Direction

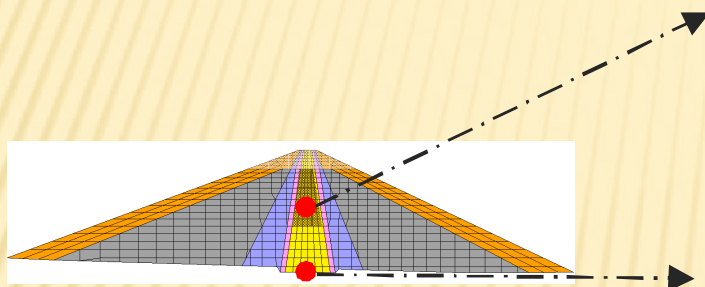


— **Record**
 — **Analysis**

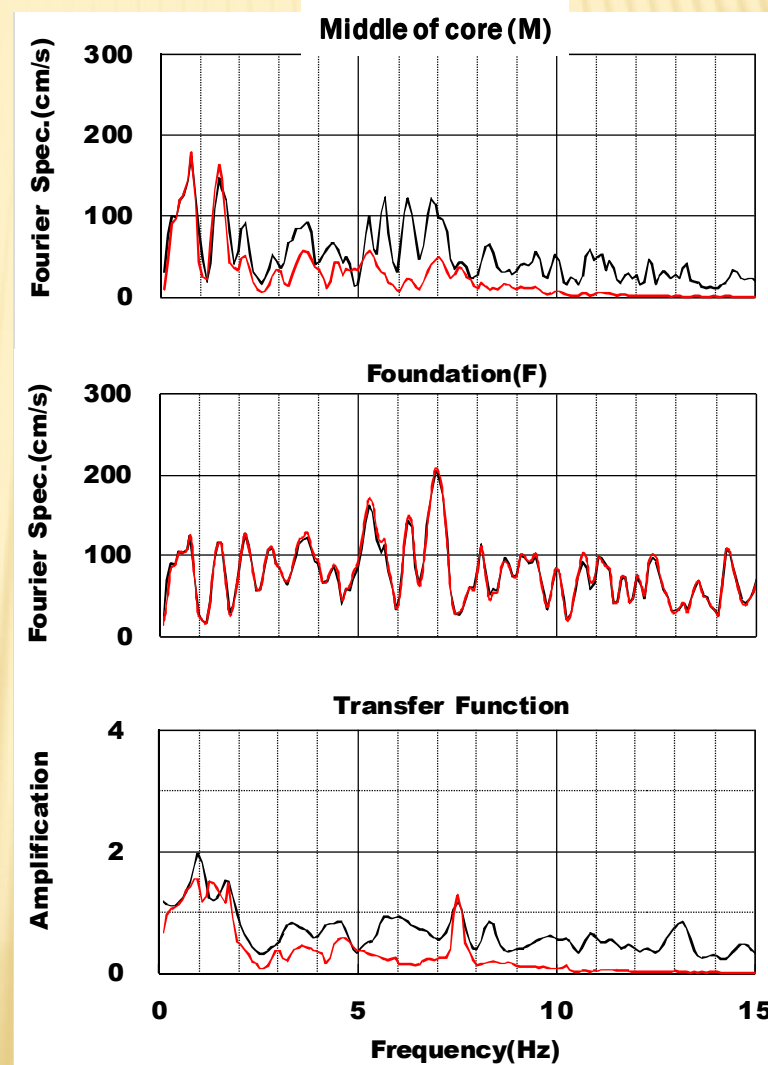


7-3 Spectra & transfer functions (5/6)

Cross Stream Direction

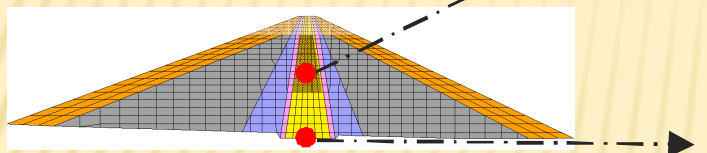


— **Record**
 — **Analysis**

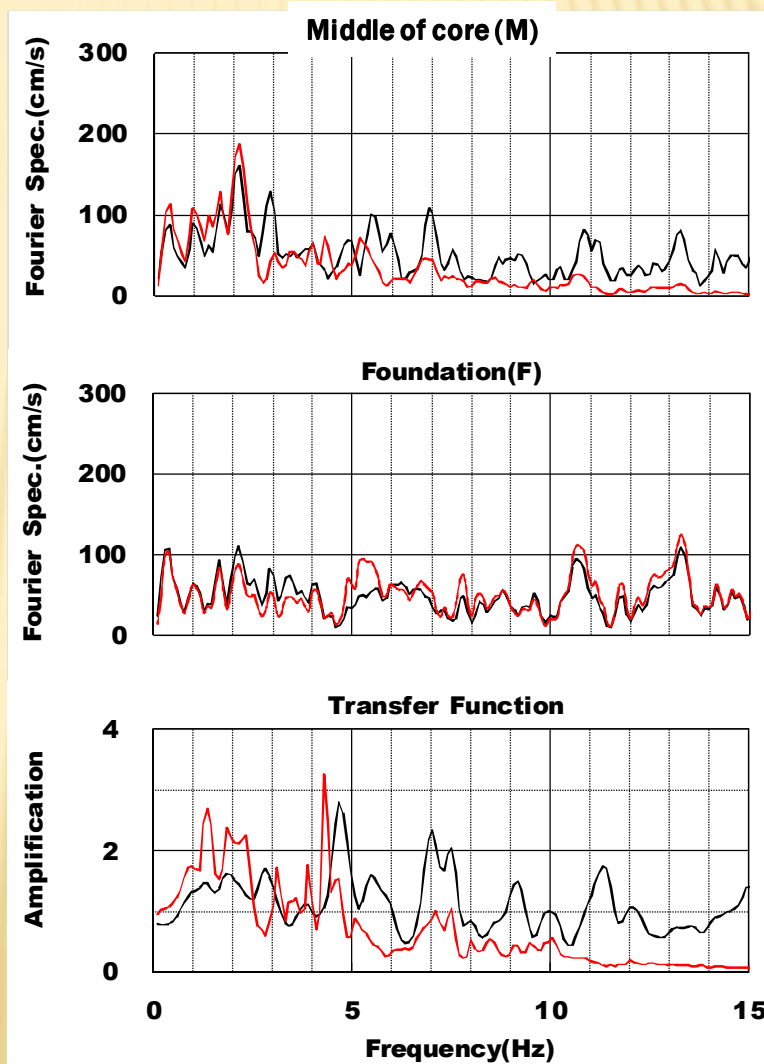


7-3 Spectra & transfer functions (6/6)

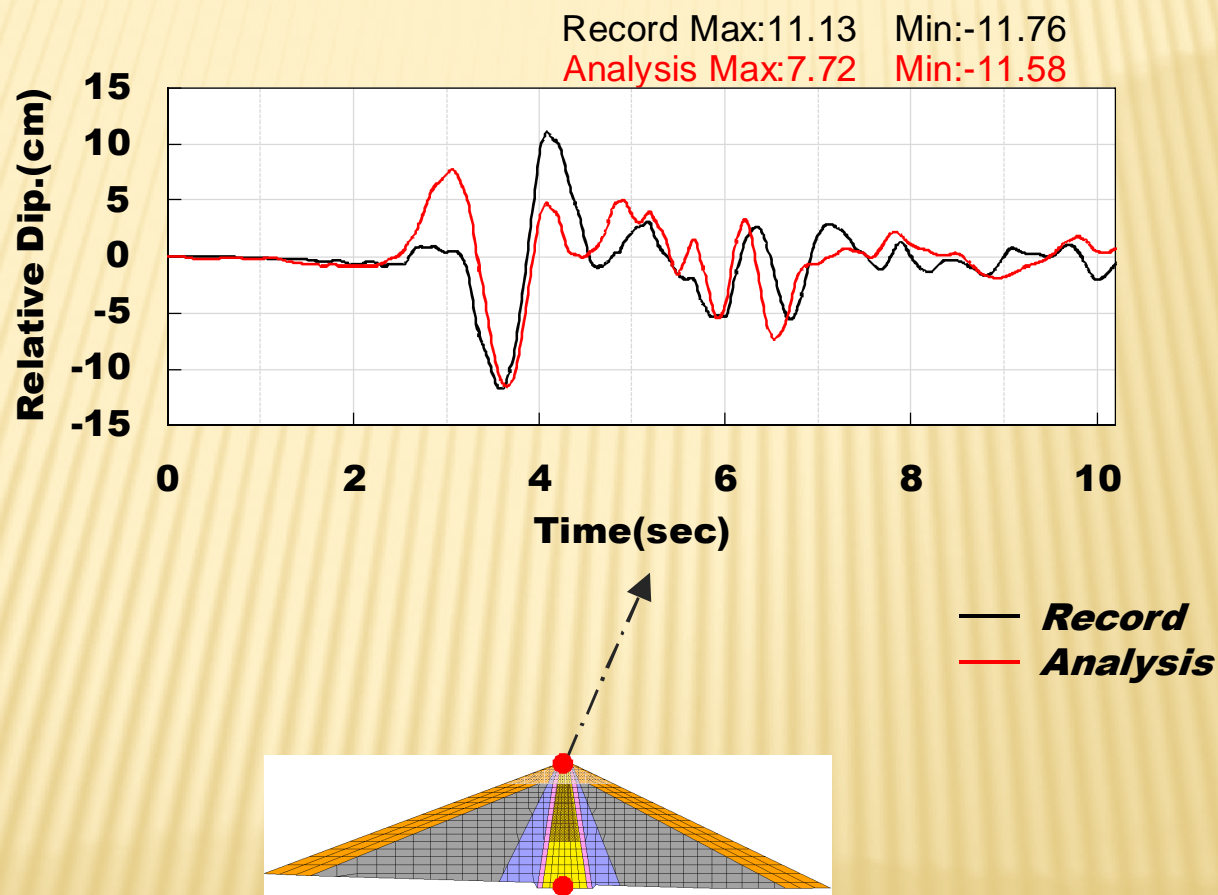
Vertical Direction



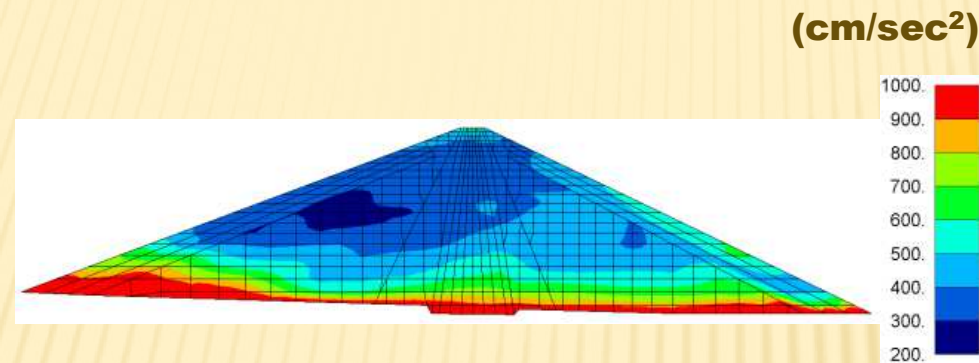
— **Record**
 — **Analysis**



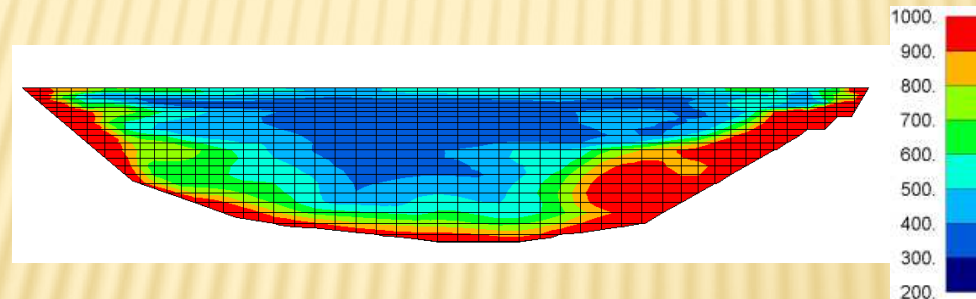
7-4 Comparison of relative displacement in stream direction



7-5 Distribution of the max. acceleration



(a) Cross Section

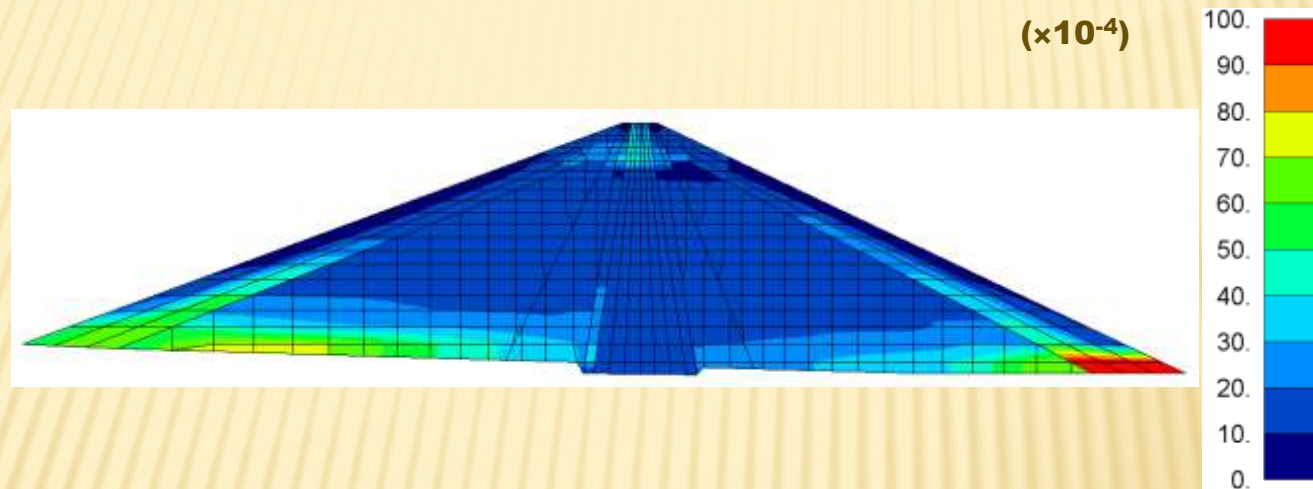


(b) Axial Section

The max. acceleration response of each nodal point in stream dir. occurred at different time.

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

7-6 Distribution of the max. shear strain

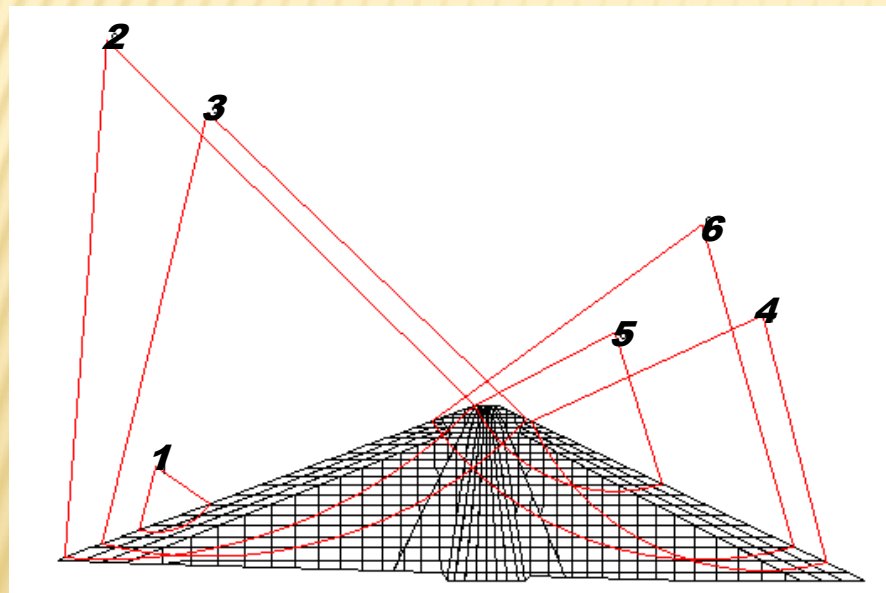
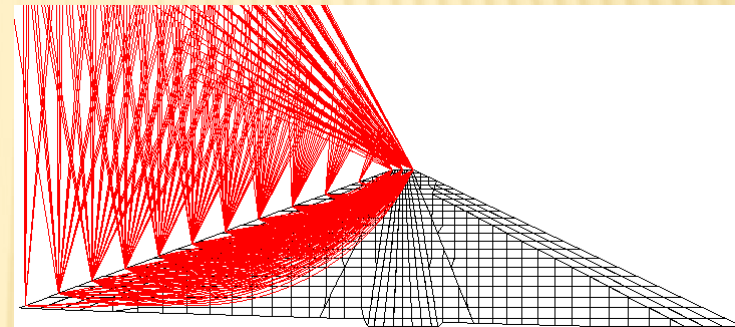


Shear strain	$\sim 10^{-4}$	$10^{-4} \sim 10^{-2}$	$10^{-2} \sim$
Phenomenon	Wave motion, vibration	Crack, settlement	Sliding, compaction, liquefaction
Mechanical characteristics	Elastic	Plastic	Fracture

The max. shear strain of each element occurred at different time.

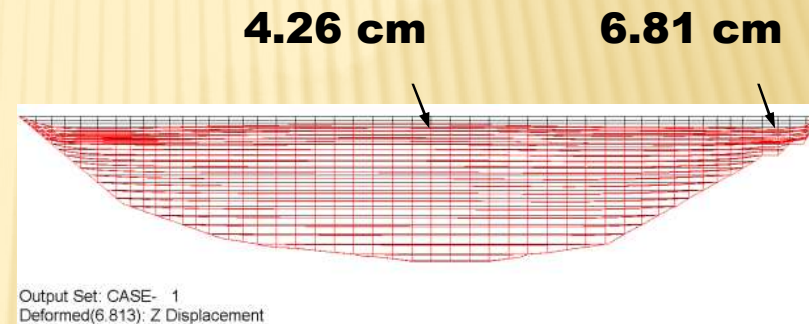
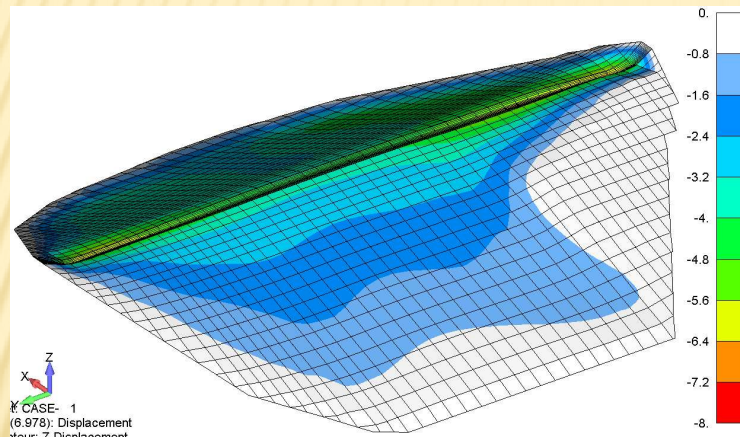
8. Mechanism of permanent deformation

8-1 Stability analysis based on circular slip surface



Sliding dir.	Arc No.	Safety Factor	Sliding Disp.(cm)
Upstream side	1	0.839	0.0006
	2	2.027	—
	3	2.395	—
Downstream side	4	2.581	—
	5	2.090	—
	6	2.135	—

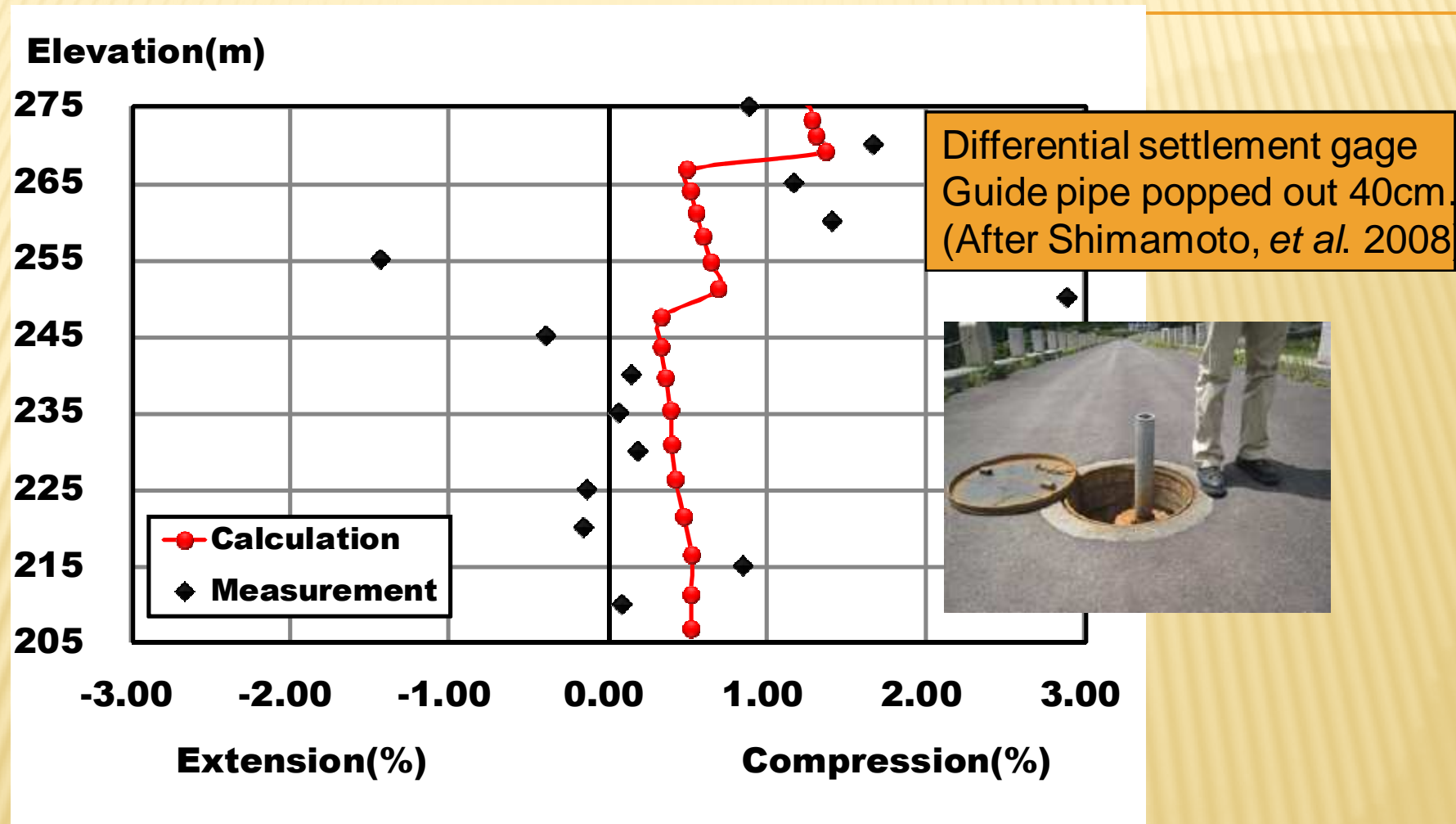
8-2 Deformation calculation based on the theory of cumulative damage



Deformation after the earthquake

$$U_r = U_a - U_b$$

8-3 Distribution of the max. shear strain



**Comparison of measured and calculated subsidence strain
(Measurement period: Dec. 4, 2007 to June 17, 2008)**

The reproduction analysis of Aratozawa dam during the 2008 Earthquake

9. CONCLUSIONS

Mechanism of the peculiar seismic behavior

Large shear strain occurred near the rock contact surface, which reduced the stiffness and increased the damping of the embankment.

The material near the crest and the slope surface became loosely during the earthquake, hence, the high frequency components lost.

Mechanism of permanent deformation

Permanent deformation was mainly due to the shaking subsidence of the soil materials. Sliding phenomenon did not occur during the earthquake.

THANK YOU
FOR YOUR
ATTENTION

